Summary of Floods in the United States during 1968

GEOLOGICAL SURVEY WATER-SUPPLY PAPER 1970-B

Prepared in cooperation with Federal, State, and local agencies



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By J. O. ROSTVEDT and others

FLOODS OF 1968 IN THE UNITED STATES

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UNITED STATES DEPARTMENT OF THE INTERIOR

ROGERS C. B. MORTON, Secretary

GEOLOGICAL SURVEY

V. E. McKELVEY, Director

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SUMMARY OF FLOODS IN THE UNITED STATES DURING 1968

By J. O. ROSTVEDT and others

ABSTRACT

This report describes the most outstanding floods in the United States during 1968. The four areas of most destructive flooding occurred on May 29-31 in northeastern New Jersey and southeastern New York, on March 18-26 in eastern Massachusetts and Rhode Island, on May 9-17 in southern Arkansas, and on July 16-20 in northeastern Iowa.

Heavy rains on May 28-30 in northeastern New Jersey and southeastern New York caused major flooding. Many peak discharges, especially in streams in New Jersey, exceeded that of the 50-year flood. Eight lives were lost because of the floods, and the Small Business Administration estimated the flood damage in New Jersey at \$133 million.

March 1968 was one of the wettest Marches of record in Massachusetts and Rhode Island, where total rainfall ranged from 7 to 12 inches. Most of this rain fell in two periods, March 12-13 and 17-19. This heavy rain caused outstanding floods on March 18-26. Peak discharges in many streams exceeded previous maximums; some were the greatest since 1886. No lives were lost in the floods, but damage was estimated at \$28 million in Massachusetts and \$9 million in Rhode Island.

Heavy rain fell in southern Arkansas on May 7-18 over an area extending from Little Rock to the southwest corner of the State. The wetting of the soil from a storm of May 9-10 was conducive to flooding following a second storm on May 13. Flooding was severe and widespread, millions of acres of land were inundated, and damage amounted to \$18.3 million.

A severe storm occurred in the central part of northeastern Iowa on the night of July 16-17. The storm caused record floods in the central part of the Wapsipinicon River basin and in tributaries of the Cedar River. Four lives were lost. A flood-control dam on Virden Creek was overtopped and the outlet structure was washed out. Damage was estimated at \$14.5 million.

In addition to the four areas of flooding mentioned above, 20 others of lesser magnitude are considered important enough to be included in this annual flood summary.

INTRODUCTION

This report summarizes information on outstanding floods in the United States during 1968. The floods reported were unusual hydrologic events in which large areas were affected, great damage resulted, or record-high discharges or stages occurred and in which sufficient data were available for the preparation of a report.

Water-Supply Paper 1970-A, "Floods of May 1968 in South Arkansas" (Gilstrap, 1972) is a special report which describes those floods in detail. The areas for which flood reports have been prepared for 1968 are shown in figure 1. The area discussed in chapter A is indicated by a dot pattern, and other areas discussed in this summary chapter are shown by a line pattern. The months in which the floods occurred are shown.

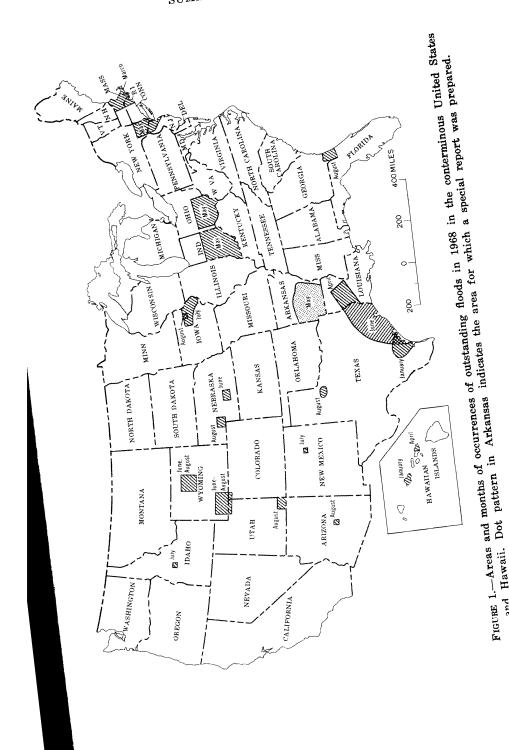
A flood may be defined as any abnormally high streamflow that overtops natural or artificial banks of a stream; every year, a great number of these events occur that are unreported.

Each flood in this report was selected as an outstanding or relatively rare event. A rare flood is not necessarily an impressive flood, but it is one whose probability of being duplicated at any one site is small. A rare flood in an isolated area or in a sparsely inhabited area could possibly be a more outstanding hydrologic event than a much publicized flood in a developed area.

Innumerable combinations of the variable factors of meteorology and physiography produce floods of all degrees and severity. Some meteorological factors influencing floods are the form, amount, duration, and intensity of precipitation; amount of previous precipitation, which would affect the moisture absorption of the soil; the temperature, which may cause frozen soil or may determine the rate of snowmelt; and the direction of storm movement. The principal physiographic features of a basin that determine flood flows are drainage area, altitude, geology, shape, slope, aspect, and vegetative cover. With the exception of vegetative cover and soil preconditioning the physiographic features are fixed for any given area. The combination of the magnitude and intensity of meteorologic phenomena, the antecedent moisture conditions, and the effect of inherent physiographic features on runoff determines what the magnitude of a flood will be.

According to U.S. Weather Bureau data, losses from floods in the United States during 1968 (\$339 million) were about 85 percent of the national annual average of \$400 million, based on the 15-year period 1951-65 and adjusted to the 1965 price index.

Nationally, the number of lives lost because of floods in 1968 was 31 compared with 34 in 1967 and 50 in 1966 and was much less than the annual average of 77 lives lost during the 44-year period, 1925-68.



Many of the flood reports give the amount of rainfall and the duration of the storm associated with the flooding. Recurrence intervals of these storms may be determined from the U.S. Weather Bureau (1961) or from a simplified set of isopluvial maps and charts contained in a report by Rostvedt (1965).

Continuing investigation of surface-water resources in the areas covered by this report is performed by the U.S. Geological Survey in cooperation with State agencies, the U.S. Army Corps of Engineers, the Bureau of Reclamation, and other Federal or local agencies. Some data were obtained from U.S. Weather Bureau publications.

Data were collected, computations were made, and most of the text was written by the district offices in whose districts the floods occurred.

DETERMINATION OF FLOOD STAGES AND DISCHARGES

The usual method of determining stream discharges at a gaging station is the application of a stage-discharge relation to a known stage. The relation at a station is usually defined by current-meter measurements through as much of the range of stage as possible. If the peak discharge at a station is above the range of the computed stage-discharge relation, short extensions may be made to the graph of relation by logarithmic extrapolation, by velocity-area studies, or by the use of other measurable hydraulic factors.

Peak discharges that are greatly above the range of the stage-discharge relation at gaging stations and peak discharges at miscellaneous sites that have no developed stage-discharge relation are generally determined by various types of indirect measurements. Adverse conditions often make it impossible to obtain current-meter measurements at some sites during major floods. Peak discharges at these sites are measured, after the floods have subsided, by indirect methods based on detailed surveys of selected channel reaches. A general description of the indirect methods used by the Geological Survey is given by Corbett and others (1943), Johnson (1936), and Dalrymple and others (1937). More detailed information concerning the latest techniques is available in recent reports by Kindsvater and others (1953), Bodhaine (1963), and Tracy (1957).

EXPLANATION OF DATA

The floods are described in chronological order. Because the

type and the amount of information differ for the floods, no consistent form can be used to report the events.

The date for each flood include: A description of the storm, the flood, and the flood damage; a map of the flood area showing flood-determination points and, for some storms, precipitation stations or isohyets; rainfall amounts and intensities; and peak stages and discharges of the streams affected.

When considerable rainfall data are available, they are presented in tabular form and show daily or storm totals. When sufficient date are available to determine the pattern and distribution of rainfall, an isohyetal map may be shown.

A summary table of peak stages and discharge is given for each flood unless the number of stations in the report is small, and then the information is included in the text description.

In the summary table the first column under maximum floods gives the period of known floods prior to the 1968 floods. This period does not necessarily correspond to that of gaging station operation, but the period may extend back to an earlier date. More than one period of known floods are shown for some stations. A period is shown whenever it can be associated with a maximum stage, even though the corresponding discharge may not be known. A second, shorter period of floods is then given in which maximums of both discharge and stage are known.

The second column under maximum floods shows the year, within the period of known floods, prior to the 1968 floods being reported, in which maximum stage or discharge occurred. The third column gives the date of the peak stage or discharge of the 1968 floods.

The last column gives the recurrence interval for the 1968 peak discharge. The recurrence interval is the average interval, in years, in which a flood of a given magnitude (the 1968 peak) will be exceeded once as an annual maximum. A flood having a recurrence interval of 20 years is one that can be expected to occur, on the average, once in 20 years or that has a 5-percent chance of occurring in any year. The recurrence intervals in the tables were obtained from U.S. Geological Survey reports on flood magnitude and frequency. In nearly all flood-frequency reports used, the data limit the determination of recurrence intervals to 50 years; in a few reports the limit is less than 50 years. The severity of a flood whose recurrence interval exceeds the limit of determination is expressed as the ratio of its peak discharge to the discharge of the flood that has a recurrence interval equal to the limits of determination.

SUMMARY OF FLOODS

FLOODS OF JANUARY 5 IN SOUTHERN OAHU, HAWAII 1

Development of deep and nearly stationary low-pressure areas at upper levels and at the surface caused cold and unstable weather conditions over the Hawaiian Islands.

Rainfall on Oahu during the night of January 4 was heavy on the leeward slopes of the Koolau Range extending from Wahiawa to the Makiki Valley area. The heaviest rain, beginning about 9:30 p.m. on the 4th and extending through the early morning hours of the 5th, fell over the central highlands of the island (fig. 2). The high intensity rainfall caused severe flooding and extensive damages to areas in Waiawa, Pearl City, and Waimalu.

The Pearl City Terrace station in Pearl City recorded 6.77

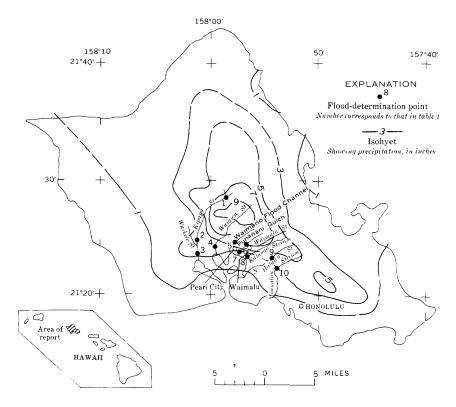


FIGURE 2.—Flood area; location of flood-determination points and isohyets for January 5, floods of January 5 in southern Oahu, Hawaii.

¹ Modified from U.S. Corps of Engineers (1968a).

inches of rain from 9:00 p.m. on the 4th through 5:00 a.m. on the 5th; 2.78 inches fell during the first $2\frac{1}{2}$ hours, and after stopping for almost 3 hours, a heavy burst dumped 3.97 inches in the last 2 hours. Of this total, slightly more than 2 inches fell between 3:15 a.m. and 3:30 a.m.; also 3.57 inches were recorded between 3:15 a.m. and 4:00 a.m. Intensities in the area have recurrence intervals of over 100 years.

Two unofficial plantation gages on higher ground above Pearl City reported 9.08 and 9.40 inches of rain for the 24-hour period ending at 8:00 a.m. on January 5. Most of the rain fell during the night and the early morning hours. Stations located in other parts of the island recorded scattered amounts of rainfall ranging from 0 to 3.31 inches. None of these stations reported any unusual amounts or intensities.

Waiawa Stream, with 16 years of record, had a discharge of 23,400 cfs (cubic feet per second). The Waimano Flood Channel had a discharge of 3,400 cfs at a site 0.5 mile upstream from the gage. Additional runoff between the measurement site and the gage did not enter the channel. Punanani Gulch, a miscellaneous measurement site tributary to Waimalu Stream, had a discharge of 4,500 cfs. Slope-area measurements were completed by U.S. Geological Survey personnel to determine peak discharges during this storm. A summary of streamflow data is presented in table 1.

Table 1.—Flood stages and discharges, January 5 in southern Oahu, Hawaii

	Stream and place of determination		Maximum floods							
No.		Drainage area	Known be January 1	January	Gage	Discharge				
		(sq mi)	Period	Year	1968	height (feet)	(cfs)			
1	Kipapa Stream near Wahiawa.	4.29	1957-68	1963	5	12.29 8.12	5,680 1,520			
2	Kipapa Stream near Waipahu.	13.8	1967-68	1967	- 5	9.08 16.5	1,090 7,300			
3	Waikele Stream at Waipahu.	45.7	1951–68	1954	5	1 14.82 9.72	13,600 7,360			
4	Waiawa Stream near Pearl City.	26.4	1 952 –68	1954	- <u>-</u> 5	$\substack{19.27 \\ 20.56}$	16,900 23,400			
5	Waimano Flood Channel at Pearl City.				5		3,400			
6	Punanani Gulch at Wajau.				5		5,400			
7	Waimalu Stream at Aiea.	6.07	1952-62, 1963-68.	1965		6.30	6,930			
			2000 001		5	6.82	8,100			
8	Kalauao Stream at Moanalua Road	2.59	1957–68	1963	5	$\frac{6.63}{3.56}$	2,580 1,200			
9	at Aiea. North Halawa Stream near	3.45	1939-33, 1953-68.	1932		13.36	6,650			
	Aiea.		555555	55.5	5	10.40	1,210			
10	Moanalua Stream near Honolulu.	2.73	1926-68	1930	5	$\frac{11.58}{7.63}$	4,580 1,160			

¹ At site 300 feet downstream at datum 1.30 feet higher.

Extremely high flow in Punanani Gulch, a tributary to Waimalu Stream, caused extensive damage to Waimalu Elementary School. The normally dry gulch, without a defined channel in the lower reaches, carried the rampaging waters through the sugarcane fields, across the highway, and slammed huge tree trunks and other debris against the school building. Water up to $4\frac{1}{2}$ feet deep smashed through six classrooms, washing furniture, books, supplies, and other equipment into the river. Three automobiles and a converted garage storing school furniture and appliances were completely demolished. Debris and mud up to 2 feet deep covered the school grounds. Damage to the school was about \$75,000. Damage to homes in the flood area was minimal owing to the high floor lines. Equipment and other property in a contractor's yard was also damaged by the floodwater. Total damage to private property amounted to about \$25,000.

Backwater created by blocked culverts in the Waimano Flood Channel caused considerable damage to public and private properties in the Pearl City area during the early morning flood. The Pearl City Shopping Center was inundated to a depth of 6 feet. Ten business establishments lost an estimated \$745,000 in damage.

Two bridges consisting of double box culverts slightly upstream from the shopping center were clogged with debris. Chain link fences were washed into the stream forming efficient barriers and nets in the culverts, stopping debris as it floated by. As a result, the floodwaters rose more than 8 feet above the 10foot-deep channel. More than 20 homes were inundated to 1-foot depth and several others, to a depth of 4-5 feet. Five homes were severely damaged when swept off their foundations. Plants, small trees, sheds, and a car were swept downstream. Water overflowed at an upper culvert sending a stream 3-4 feet deep rushing down a two-block long street and washing everything away within its path. Three other cars were destroyed, and two large trailer dump trucks were swept more than 100 feet down the street. One home near the culvert was lifted from the foundation and turned around by the force of the water. A private childeren's day care center lost \$5,000 worth of school equipment, furniture, and supplies.

Farther upstream, another blocked culvert sent water over the road and through two more homes, which caused a considerable amount of damage. The estimated damages to residential property in the Pearl City area were about \$140,000.

Water over the right bank of the channel upstream from the

Noelani Street culvert undermined and destroyed 450 feet of the rubble masonry wall. Damages to the Waimano Flood Channel totaled about \$158,000. A concrete-lined channel at the Pacific Palisades Drainageway in the newly developed Pacific Palisades subdivision was damaged during the storm.

Damage in the lowland areas of Waiawa was extensive. Seven homes near the Ewa-Schofield Junction at Kamehameha Highway were totally destroyed, and 12 others were severely damaged by high water. The river rose more than 15 feet above the banks, carried the homes more than 400 yards downstream, and crushed them together near the highway bridge. Banana fields were leveled, many head of livestock were drowned, and huge quantities of household goods and furniture were lost.

Areas downstream from the Kamehameha Highway bridge were flooded. The rampaging stream went over the low banks again and damaged a home and an automobile. A sewage treatment plant was also damaged. The low-lying area, nearly at sea level, is sparsely populated and used mainly for agriculture. Nine acres of crops were destroyed and a number of cattle, hogs, and poultry were lost.

Damages to residences in Waiawa totaled about \$69,000; and agricultural losses, \$33,600.

FLOODS OF JANUARY 18-24 IN SOUTH-CENTRAL TEXAS

By L. G. STEARNS

Frequent rains, accompanied by an occasional intense rainstorm, made the month of January one of the wettest Januarys on record at many weather stations in south-central Texas.

The U.S. Weather Bureau at San Antonio recorded a monthly rainfall of 8.52 inches, the greatest amount ever recorded in January at that station since records began in 1871. Above-normal rainfall occurred during the month at nearly all stations in the area. During the period January 18–21, heavy rains of 10 or more inches caused flooding from San Antonio southward to the Gulf Coast (fig. 3). During the period January 18–24, the basins of the Guadalupe and Nueces Rivers and their tributaries were the principal basins affected by flooding (table 2). Several adjacent smaller basins were affected to a lesser extent. Although record rainfall totals were established during January at several stations, floods were not the maximum of record at any gaging station during this period.

Flash flooding in San Antonio caused five deaths and property damage estimated at \$4 million. Bexar County was declared a disaster area for the first time in history.

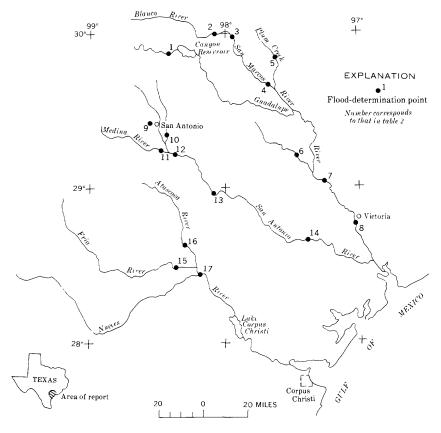


FIGURE 3.—Flood area; location of flood-determination points, floods of January 18-24 in south-central Texas.

Table 2.—Flood stages and discharges, January 18-24 in south-central Texas

			Maximum floods						
		.	77				Disch	arge	
No.	Stream and place of determination	Drainage area	Known be January		January 1968	Gage height (feet)	Cfs	Recur- rence interval (years)	
NO.	or determination	(sq mi)	Period	Year					
			Guadalupe Riv	ver basir	1				
1	Guadalupe River near Spring Branch.	1,282	1859-1968 1922-68	1869 1932	20	53 42.10 17.3	(¹) 121,000 13,600	2	
2	Blanco River at Wimberley.	364	1869-1968	1929	20	31.10 12.90	113,000 18,200	- 4	
3	Blanco River near Kyle.	424	1882-1968	1929	20	40 23.33	139,000 26,600	- 5	
4	San Marcos River at Luling.	833	1859-1968 1939-68	$1869 \\ 1952$		40.4 34.95	(¹) 57,000		
5	Plum Creek at Lockhart.	113	1905–68 1959–68	1936 1960	21 21	31.42 22 20.62 15.92	24,800 (1) 26,600 4,350	3 	

Table 2.—Flood stages and discharges, January 18-24 in south-central Texas—Continued

					Maximum	floods		
	Stream and place of determination	Drainage area	Known be			Gage	Disch	arge Recur-
No.		(sq mi)	Period	Year	January 1968	height (feet)	Cfs	rence interval (years)
		Guada	lupe River bas	in—Cor	ntinued			
6	Sandies Creek	560	1864-1968	1936		33.1	92,700	
ь	near Westhoff.	900	1804-1908		22	22.82	6,550	< 2
_		4 0==						_
7	Guadalupe River	4,877	1900-68	1936		44.33	(1)	
	at Cuero.		1903-06, 1917-68.	1929	-	³ 35.2	101,000	
					23	33.81	44,200	6
							179,000	
8	Guadalupe River	5,161	18331968	1936		31.22	179,000	
-	at Victoria.	-,			25	29.72	44,300	6
9	San Antonio	42	1819-1968	1921		3 20.14	15,300	
,	River at San Antonio.	44	1019-1900	1921	18	11.23	5,730	8
10	Salado Creek	189	1941-68	1946		26.8	(¹)	
10	(lower station)	109	1941-00	1960		26.8	(1)	
			1001 00			22.11		
	at San Antonio.		1961-68	1965	10		6,090	
				2222	18	22.88	6,630	3
11	Medina River	1,317	1 912 –68	1946	5.5	39.23	31,800	- 1
	near San Antonio.				18	28.56	13,100	< 2
12	San Antonio River	1,743	1900-1968	1946		61	(¹)	_
	near Elmendorf.	•	1963-68	1965		35.34	15,100	_
					18	40.15	29,800	25
13	San Antonio River	2,113	1875-1968	1946		33.80	47,400	
10	near Falls City.	2,110	1010-1000		21	22.87	17,300	8
14	San Antonio River	3,921	1869-1968	1967		53.7	138,000	· ·
14	at Goliad.	5,921	1909-1909	1907	24	41.98	25,900	8
			Nueces River	basin				
15	Frio River at Calliham.	5,491	1870–1968	1932	21	39.2 29.84	109,000 12,700	3
16	Atascosa River	1.171	1881-1968	1967		41.3	121,000	•
10	at Whitesett.	1,111	1001-1900		21	35.93	23,500	18
17	Nueces River near	15 000	1005 1000	1967	21	49.21	141.000	10
TA		15,600	1875–1968		ōō			6
	Three Rivers.				22	40.64	36,000	ь

Not determined.

FLOODS OF MARCH 18-26 IN EASTERN MASSACHUSETTS AND RHODE ISLAND²

Precipitation during March in eastern Massachusetts and Rhode Island was considerably above normal. The U.S. Weather Bureau reported that, in much of the area, March 1968 was one of the wettest Marches of record with monthly totals as high as 7 to 12 inches. Most of the precipitation fell during two storms the first on March 12-13, the second on March 17-19. Precipitation during the first storm was generally much less than that during the second, but with runoff from snow still remaining in sheltered areas, the earlier storm contributed to the severe flooding. In addition, the time of the year—prior to the start of the growing season—increased the magnitude of the flood peaks and volume of runoff.

² At site 548 feet upstream. ³ At former site and datum.

² Modified from G. K. Wood, L. A. Swallow, C. G. Johnson, and G. H. Searles (1970).

The Weather Bureau reported that at many stations new 24-hour precipitation records were established for March. Blue Hill Observatory at Milton, Mass., reported a 24-hour total of 6.62 inches on March 17 and 18. An isohyetal map for Massachusetts and Rhode Island, adapted from a map prepared by the Weather Bureau, is presented in figure 4.

The flooding of March 18–26 was outstanding. Many streams with long-term streamflow records had flood peaks which exceeded previous maximums (table 3). For Ipswich River near Ipswich, Mass., (No. 19, table 3) the March peak flow exceeded

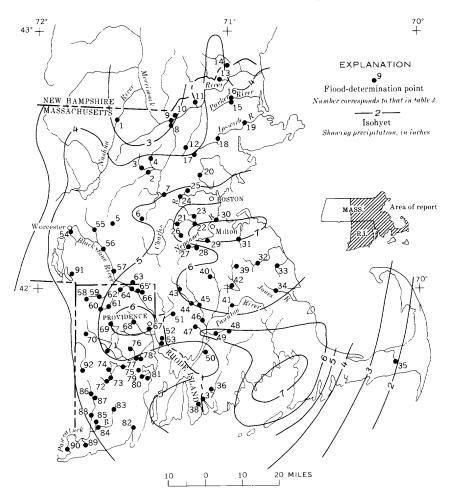


FIGURE 4.—Flood area; location of flood-determination points and isohyets for March 17-19, floods of March 18-26 in eastern Massachusetts and Rhode Island.

Table 3.—Flood stages and discharges, March 18-26 in eastern Massachusetts and Rhode Island

					Maximu	ım floods		
	Stream and place of determination	Drainage	e Known before			**	Discha	rge
No.		area (sq mi)	March Period		March - 1968	Gage height (feet)	Cfs	Recur- rence interva (years)
		М	errimack Riv	er basin	ı			
1	Nashua River at East Pepperell, Mass. ¹	1 316	1936-68	1936	20	19.1 11.7	² 20,900 6,900	(*)
2	Assabet River at Maynard, Mass.	116	1887–1968	$1955 \\ 1955$	19	4 8.96 8.15	4,250 3,620	⁵ 1.1
3	Heath Hen Meadow Brook	3.89	1964-68	1967	18	5.41 9.80	54 156	(3)
4	at Stow, Mass. Nashaba Brook near Acton,	12.7	1964-68	1967	19-20	$\substack{4.11\\5.07}$	128 360	(3)
5	Mass. Jackstraw Brook at Westboro,	1.11	1964-68	1964	18	11.95 12.72	27 27	(³)
6	Mass. Sudbury River at Framingham	75.2	1875-1968	1955	18	8.47	6 3,650 6 2,000	
7	Center, Mass. Hayward Brook at Wayland,	2.86	1964-68	1965	18	7.57 8.47	48 74	(3)
8	Mass. Concord River below River Meadow Brook	⁷ 3.12	1937-68	1955	22	8.97 9.16	84,540 84,800	- (³)
9	at Lowell, Mass. Merrimack River below Concord River at	9 4,425	1736–1968	1936	23	68.4 51.15	10 173,000 10 44,400	-
10	Lowell, Mass. Richardson Brook near Lowell,	1,32	1962-68	1962	18	14.68 14.44	153 186	(³)
11	Mass. Spicket River at	73.8		_	19		1,440	(3)
12	Methuen, Mass. Shawsheen River near Wilming-	3.51	1964-68	1967	19	6.72 8.60	495 1,050	(³)
13	ton, Mass. East Meadow River near	1.93	1962-68	1962	19	5.6 5.45	190 211	(₃)
14	Haverhill, Mass. Cobbler Brook near Merrimac, Mass.	.77	1962-68	1962	18	9.30 10.69	66 117	- (³)
			Parker River	basin				
15	Parker River tributary near Georgetown,	.65	1964-68	1965 1966	18	10.90	24 76	(3)
16	Mass. Parker River at Byfield, Mass.	21.6	1946-68	1958	19	5.99 5.61	479 489	25
			Ipswich River	basin				
17	Maple Meadow Brook at Wilmington,	3.99	1962-68	1962	19	5.33 5.63	103 119	(8)
18	Mass. Ipswich River at South Middle-	43.4	1937-68	1962	19	$\frac{6.99}{7.09}$	808 833	45
19	ton, Mass. Ipswich River near Ipswich, Mass.	124	1887-1968	1936	20, 21	7.70 8.41	2,610 2,680	50
			Mystic River	basin			- W W W W W W W W	
20	Aberjona River at Winchester, Mass.	23.3	1887–1968	1955 1962	19	11 14.43 13.73	835 649	(3)

Table 3.—Flood stages and discharges, March 18-26 in eastern Massachusetts and Rhode Island—Continued

					Maximu	ım floods		
		Drainage	Known b	efore			Discha	rge
No.	Stream and place of determination	area (sq mi)	March 1		March 1968	Gage height (feet)	Cfs	Recur- rence interval (years)
			Charles River	basin				
21	Charles River at Charles River Village, Mass.	184	1887–1968	1955	21-22 22	9.24 8.72	3,220 3,220	40
22	Mother Brook at		1932-68	1955	21	92.90	970 1,040	-
23	Dedham, Mass. Charles River at	211	1960-68	1960		87.18 5.16	1,470	
24	Wellesley, Mass. Charles River at Waltham, Mass.	12 227	1887-1968	1936 1955	21 22	6.20 5.35 4 5.39	2,410 2,540 (3)	(8)
25	Beaver Brook at Belmont, Mass.	4.09	1962-68	1962	18	12.46 13.07	200 212	(3)
		N	eponset Rive	r basin				
26	Mill Brook at Westwood, Mass.	1.52	1964-68	1965 1967	10	7.05 7.05	31 31	(3)
27	Neponset River at	35.2	1887-1968	1955	18	10.12 14.65	96 1,490	
28	Norwood, Mass. East Branch	27.2	1953-68	1955	18	10.48 8.18	$\frac{1,140}{1,790}$	40
	Neponset River at Canton, Mass. Plantingfield Brook				18	6.87	1,420	25
29	at Norwood, Mass.	1. 52	1964-68	1967	18	14.11 15.06	145 178	(³)
30	Neponset River at Hyde Park, Mass.	97.8			20		1,550	(₃)
		Weyn	nouth Back F	liver ba	sin			
31	Old Swamp River near South	4.29	1967-68	1967 1967		4.22	207	
	Weymouth, Mass.				18	5.22	566	(8)
			North River	basin				
32	Indian Head River at Hanover, Mass.	30.3	1967-68	1967	18–19	5.62 7.13	788 1,390	(3)
			South River	basin				
33	Furnace Brook near Marshfield, Mass.	1.61	1964-68	1967	18	9.41 9.92	54 71	(3)
			Jones River	basin				
34	Jones River at Kingston, Mass.	¹³ 15.8	1967-68	1967		14 4 65	35 8	
	Kingston, Wass.			1967	19	14 4. 65 14 4. 60	¹⁵ 575	(3)
			Herring River	basin				
35	Herring River at North Hard- wich, Mass.	9.4	1967-68	1967	21	4.88 4.03	54 44	(3)
		East Br	anch Westpor	t River	basin			
36	Kirby Brook near head of West- port, Mass.	3.69	1964-68	1967	18	10.07 12.16	213 378	(3)

Table 3.—Flood stages and discharges, March 18-26 in eastern Massachusetts and Rhode Island—Continued

					Maximu	ım floods		
		Drainage	Known	hoforo		_	Discha	arge
No.	Stream and place of determination	Drainage area (sq mi)	March		March 1968	Gage height	Cfs	Recur rence interva
			reriou	ı ear		(feet)	Cis	(years)
		West Br	anch Westpo	rt River	basin			
37	Adamsville Brook at Adamsville, R.I.	8.6	1941-68	1960	18	7.14 7.70	273 221	6
	_		Cold Brook	basin				
38	Cold Brook near Adamsville, R.I.	1.15	1966-68	1967	18	8. 93 11.19	47 79	(3)
		7	Taunton Rive	r basin				
39	Meadow Brook tributary near Whitman, Mass.	1.04	1967-68	1967	18	9.73 10.96	49 56	(3)
40	Dorchester Brook near Brockton, Mass.	4.67	1962-68	1962 1965	18	¹⁶ 3.75 5.86	130 359	(3)
41	Taunton River at State Farm,	260	1930–68	1955	20	13.02 14.48	4,010 4,980	⁵ 1.2
42	Mass. Snows Brook near Bridgewater,	1.41	1964-68	1964	18	$\begin{array}{c} 5.75 \\ 10.09 \end{array}$	29 153	(3)
43	Mass. Wading River at West Mansfield,	19.2	1954-68	1955	19	6.22 6.60	519 541	- (3)
44	Mass. Wading River near Norton,	42.4	1926-68	1955	19	$10.98 \\ 11.47$	1,170 1,460	⁵ 1.2
45	Mass. Threemile River tributary near Oakland, Mass. Threemile River	.50	1964-68	1967	18	$\frac{6.58}{7.13}$	14 18	(3)
46	at North	83.8	1967-68	1967	19	$6.31 \\ 8.30$	1,340 2,490	(³)
47	Dighton, Mass. Segreganset River near Dighton, Mass.	10.6	1967-68	1967	18	5.81 7.51	51 3 867	(3)
48	Holloway Brook near Myricks,	1.16	1967-68	1967	18	$\frac{6.93}{7.47}$	68 8 6	(3)
49	Mass. Quaker Brook near Myricks,	1.94	1967-68	1967	18	10.02 10.78	156 190	(<u>a</u>)
50	Mass. Taunton River tributary near Fall River, Mass.	.25	1964-68	1966	18	11.06 11.23	49 53	(³)
			Palmer Rive	r basin				
51	Bliss Brook near Rehoboth, Mass.	4.96	1963-68	1967	18	5.04 5.43	251 441	(3)
]	Runnins Rive	er basin				
52	Runnins River at Seekonk, Mass.	4.24	1967–68	1967	18	6.50 7.50	94 155	(3)
		M	etacomet Bro	ok basi	n			
53	Metacomet Brook at East Providence, Mass.	.82	1966-68	1967	18	11.05 8.11	225 81	(3)
		В	lackstone Ri	ver basir	1			
54	Kettle Brook at Worcester, Mass.	31.3	1924-68	1955	18	12.78	3,970 6 1,100	

Table 3.—Flood stages and discharges, March 18-26 in eastern Massachusetts and Rhode Island—Continued

					Maximu	m floods		
		Drainage	Known b	efore			Discha	rge
No.	Stream and place of determination	area (sq mi)	March 1 Period	968 Year	March 1968	Gage height (feet)	Cfs	Recur- rence interva (years)
		Blackstor	ne River basi	in—Cor	ıtinued			(years,
55	Quinsigamond	25.5	1940-68	1955		5,15	820	
-	River at North Grafton, Mass.				19	4.03	452	(8)
6	Blackstone River at Northbridge, Mass.	139	1901–68	1 955 1955	19	4 17.53 10.90	16,900 4,760	- (³)
57	West River below West Hill Dam, near Uxbridge, Mass.	27.9	1962-68	1962	26	3.34 3.45	355 370	(8)
8	Dry Arm Brook near Wallum	1.74	1966-68	1967	18	8.30 9.72	56 92	(a)
9	Lake, R.I. Nipmuc River near Harrisville,	16.0	1965-68	1967	18	$6.58 \\ 7.42$	527 1,020	(3)
0	R.I. Chepachet River at Chepachet,	17.4	1965-68	1967	18	7.27 8.85	371 1,080	(3)
31	R.I. Mowry Paine Brook near	1.87	1966-68	1967	18	8.59 10.24	107 360	(³)
62	Chepachet, R.I. Branch River at Forestdale, R.I.	91.2	1887-1968	1936	18	11.90	5,800 4,980	⁵ 1.1
3	Bungay Brook near Sheldonville,	2.66	1964-68	1967	18	6.99 8.24	92 188	(3)
64	Mass. Blackstone River at Woonsocket. R.I.	416	1946-68	1955	19	21.8 14.64	17 32,900 15,400	25
5	Blackstone River tributary at Woonsocket,	2.22	1966-68	1967	18	2.97 3.52	116 285	(³)
66	R.I. Blackstone River tributary No. 2 at Berkeley, R.I.	1.04	1966-68	1967	18	6.09 5.80	34 27	(8)
		M	oshassuck Riv	er basi	n			
67	Moshassuck River	23.1	1964-68	1967		4.40	1,110	
68	Providence, R. I. Woonasquatucket River at	38.3	1936, 1942–68.	1954	18	4.34 7.03	2,390 1,100	(3)
	Centerdale, R.I.				18	7.75	1,440	⁵ 1.1
		F	awtuxet Rive	r basin				
69	Mosquitohawk Brook near North Scituate,	3.06	1966-68	1967	18	3.09 3.40	29 3 630	(3)
70	R.I. Shippee Brook tributary at North Foster,	.55	1966-68	1967	18	8.74 9.87	21 38	(8)
71	R.I. Wilbur Hollow Brook near Clayville, R.I.	4.61	1966-68	1967	18	12.20 14.40	128 300	(³)
2	Nooseneck River at Nooseneck	8.23	1964-68	1965	18	4.65 6.13	223 318	(3)
73	R.I. Carr River near	6.73	1964-68	1965	=====	4.77	96	
74	Nooseneck, R.I. Bear Creek near	3.98	1966-68	1967	18	$6.50 \\ 8.59$	221 110	(3)
75	Coventry, R.I. South Branch Pawtuxet River	63.8	1936, 1941–68.	1936	18	10.11	118 1,810	(3)
	at Washington, R.I.		1941-00.		18-19	5.13	1,860	(3)

Table 3.—Flood stages and discharges, March 18-26 in eastern Massachusetts and Rhode Island-Continued

			Maximum floods							
				_			Discharge			
Nο.	Stream and place ar	Drainage area (sq mi)	Known before March 1968		March 1968	Gage height		Recur- rence		
			Period	Year		(feet)	Cfs	interval (years)		
		Pawtux	et River basin	-Cont	inued					
76	Furnace Hill Brook at	4.19	1966-68	1967	18	4.58 4.67	516 586	(⁸)		
77	Cranston, R.I. Pawtuxet River at Cranston, R.I.	200	1940-68	1967	18–19	$9.95 \\ 11.53$	2,480 3,110			
78	Pocasset River near North Scituate, R.I.	1.34	1966-68	1966	18	7.63 9.29	15 55	(³)		
		***************************************	Hardig River	basin						
79	Hardig Brook near West Warwick, R.I.	3.16	1966-68	1967	18	4.49 5.49	160 300	(3)		
		Po	towomut Riv	er basiı	n					
80	Frenchtown Brook near	3.12	1966–68	1966	18	4.16 6.86	41 118	(³)		
81	Davisville, R.I. Potowomut River near East Greenwich, R.I.	23.0	1938–68 1941–68	1938 1962	18	¹⁸ 8.5	482 866	⁵ 1.2		
			Smelt Brook							
82	Browns Brook at	.50	1966-68	1966		8.15	11			
	Wakefield, R.I.				18	10.53	19	(⁸)		
		P	awcatuck Riv	er basiı	1					
83	Glen Rock Brook tributary at	1.04	1966-68	1966	18	5.32 6.51	57 104	(³)		
84	Usquepaug, R.I. Pawcatuck River at Wood River Junction, R.I.	100	1941–68	$^{1954}_{1962}$	19	6.23 7.80	1,130 1,700	5 1.1		
85	Meadow Brook near Carolina, R.I.	5.53	1966-68	1967	18	4.25 6.07	55 164	(³)		
86	Wood River near Arcadia, R.I.	35.2	1964-68	1964	18	(³) 8.64	390 896	(3)		
87	Wood River tributary near Arcadia, R.I.	.77	1966-68	1967	18	8.05 8.87	11 19	(3)		
88	Wood River at Hope Valley, R.I.	72.4	1711-1968 1936, 1942-68.	1886 1936		¹⁹ 12.4	(³) 1,540			
89	Perry Healy Brook near Bradford,	1.82	1966-68	1967	18-19 18	$8.26 \\ 8.15 \\ 10.52$	1,720 47 108	45 (3)		
90	R.I. Pawcatuck River at Westerly, R.I.	295	1635-1968 1887-1968	1938 1927		¹⁸ 15.0	(3) (20)			
					18	10.49	4,470	5 1.1		
			Thames River							
91	Browns Brook near Webster, Mass.	.49	1963-68	1967	18	2.01 2.46	19 125	(3)		
92	Bucks Horn Brook at Greene, R.I.	5.52	1966-68	1967	18	$3.76 \\ 5.49$	144 353	(3)		

Net above gage; total above gage, 433 sq mi (flow diverted from 117 sq mi for use of Boston metropolitan district and city of Worchester).
 Includes water wasted in diverting drainage from 117 sq mi in basin of South Branch Nashua River.
 Not determined.
 Affected by backwater from debris.
 Ratio of peak discharge to 50-year flood.
 Maximum daily.

that of the great winter flood of 1886. At Charles River Village, Mass., on the Charles River (No. 21, table 3) the March peak equaled that of August 1955, both of which were greater than any other since at least 1886. Many streams reached peaks having recurrence intervals greater than 50 years. Streams such as Wading and Taunton Rivers in southeastern Massachusetts and Potowomut and Pawcatuck Rivers, among others in Rhode Island, had peak flows of this magnitude.

Damage from the flooding was great, but there was no loss of life. Thousands of homes suffered damage, and industrial, commercial, and public losses were severe. The Corps of Engineers estimated that losses in Massachusetts amounted to about \$28 million and in Rhode Island, \$9 million. Damage to dwellings exceeded all other losses. The Corps indicated that the major factor in the size of residential losses was the development of wetland areas over the years. Encroachment of stream channels in urban centers was also a contributing factor in overall losses.

FLOODS OF APRIL 8-11 IN WEST-CENTRAL LOUISIANA

By Fred N. Lee

An almost stationary line of thunderstorms from 50 to 75 miles wide caused widespread flooding in west-central Louisiana on April 8-11. Two cells of intense rainfall occurred along this line of thunderstorms, one in the Many-Robeline area and the other in the Montgomery area (fig. 5). Unofficial reports from Burmuda, near Montgomery, and from Fort Jesup, near Many, indicated rainfall of 14.0 inches, and the U.S. Weather Bureau tion at Many reported rainfall of 12.62 inches. Several other stations reported rainfall in excess of 8.0 inches.

At five discharge stations and at two crest-stage stations the peak discharge exceeded the known maximum for the period of record. At five of these stations the peak discharge exceeded the 50-year flood. Table 4 is a list of the stations in west-central Louisiana where the discharge equaled or exceded a 4-year flood.

Flood damage was estimated at \$958,000. About 80,000 acres

⁷ Net above gage; total above gage, 405 sq mi.

⁸ Includes water wasted from 92.6 sq mi in basins of Sudbury River and Lake Cochituate.

⁹ Net above gage; total above gage, 4.635 sq mi.

¹⁰ Includes water wasted from 210 sq mi in basins of Sudbury and South Branch Nashua Rivers and Lake Cochituate.

¹¹ Affacted by bodywater from Thom Munita Lake.

¹¹ Affected by backwater from Upper Mystic Lake.
¹² Excludes 23.6 sq mi drained by Stony Brook, from which flow is diverted for municipal supply of Cambridge.

¹³ Excludes 4.09 sq mi above outlet of Silver Lake.
14 Affected by backwater from tide.
15 Includes water wasted from 4.09 sq mi above outlet of Silver Lake.
16 Affected by ice jam.
17 Affected by failure of dam on Mill River.

¹⁸ Affected by backwater from hurricane tidal wave. 19 At least.

²⁰ Possibly more than 6,300 cfs.

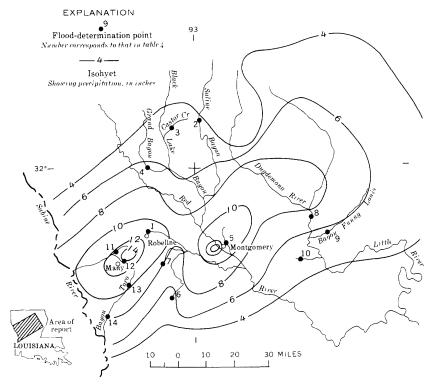


FIGURE 5.—Flood area; location of flood-determination points, and isohyets for April 8-10, floods of April 8-11 in west-central Louisiana.

in low-lying areas were inundated. Many crops were washed out or drowned and planting was delayed. Some bridges on rural roads were washed out and main highways were closed. About 150 families were evacuated in Natchitoches, Sabine, and Winn Parishes.

Table 4.—Flood stages and discharges, April 8-11 in west-central Louisiana

	Stream and place of determination		Maximum floods						
		Drainage area (sq mi)	***				Discharge		
No.				Known before April 1968		Gage		Recur-	
No.			Period	Year	1968	height (feet)	\mathbf{Cfs}	rence interval (years)	
			Red River	basin					
1	Bayou Dupont near Robeline.	35.1	195768	1961	8	10.24 11.70	5,300 13,000	¹ 1,5	
2	Saline Bayou near Lucky.	154	1940-68	1945	9	12.9 9.49	13,500 5,080	5	
3	Castor Creek at Castor.	27.9	1954-68	1955	8	48.70 48.23	$^{(2)}_{2,250}$	12	

Table 4.—Flood stages and discharges, April 8-11 in west-central Louisiana— Continued

			Maximum floods							
	Stream and place of determination	Drain a ge	V	hafana			Discharge			
No.		area (sq mi)	Known before April 1968		April 1968	Gage height		Recur- rence		
110.		(sq mi)	Period	Year	1900	(feet)	Cfs	interval (years)		
		Red	River basin-	-Contin	ued					
4	Grand Bayou	93.9	1957-68	1958		11.47	7,920			
	near Coushatta.				9	10.57	5,200	12		
5	Nantachie Creek	47	1942-68	1953		14.63	10,500			
	near Montgom-			-	8	16.38	13,800	¹ 2.09		
6	ery. Little Sandy Creek	21.4	1949-68	1953		15.36	5.880			
Ü	at Kisatchie.	2111	1010 00	1000	10	12.55	4,140	10		
7	Horsepen Creek	5.27	1949-68	1953		11.88	1,910			
	near Provencal.				8	11.49	2,040	16		
8	Little River near	1,880	1958-68	1966	_	39.22	44,700	-		
	Rochelle.				11	39.88	45,300	8		
9	Bayou Funny	92	1939-68	1953		23.26	32,700			
	Louis near				8	18.50	6,310	8		
	Trout.	_								
10	Big Creek at	51	1942-68	1953		18.03	23,500			
	Pollock.				9	13.68	6,080	26		
			Sabine Rive	r basin						
11	Harpoon Bayou	3 4 1.6	1951-68	1961		14.13	7,850			
	at Many.	-			8	16.61	23,800	1 2.43		
12	Blackwell Creek	3.16	1960-68	1961		13.20	865			
	at Many.				8	12.33	812	6		
13	Bayou Toro near	78.6	1950-68	1953		50.54	18,500			
	Florien.		55555555	- 22:2	8	50.95	(²)			
14	Bayou Toro	148	1956 - 68	1961		24.24	20,500	1100		
	near Toro.				8	25.73	31,200	¹ 1.39		

¹ Ratio of peak discharge to 50-year flood. ² Not determined.

FLOODS OF APRIL 16 IN EASTERN MAUI, HAWAII³

The weather over the Hawaiian Islands prior to April 14 was relatively dry. On April 14, a surface low-pressure trough. east of Hawaii, moved slowly westward towards the islands. This trough brought cloudy conditions and frequent showers to the State. On the morning of the 15th, a high-pressure ridge at upper levels appeared to be almost stationary over the islands. Another upper level low-pressure area was to the west of the island chain and was moving slowly eastward. Considerable high- and middlelevel clouds were present. The displacement of the high-pressure ridge by the trough, together with the surface low, brought the heavy rains over eastern Maui on April 15. The heavy rain caused large amounts of damage to the Hana district (fig. 6). Other parts of the State also received some heavy rainfall, but no serious damage resulted. Hana Airport measured 17.57 inches of rain for the 24-hour period ending at 8:00 a.m. on April 16. Hana station measured 16.0 inches for the same period. Of this total, 15.96 inches fell between 4:00 p.m. on April 15 and 7:00 a.m. on

³ During floods only. Drainage area 22.7 sq mi during low water and medium flows.

³ Modified from U.S. Corps of Engineers (1968b).

April 16. Slightly more than 1.80 inches fell within 15 minutes from 9:30 p.m. This intensity has a recurrence interval of over 100 years based on the rainfall-frequency atlas of the Hawaiian Islands (U.S. Weather Bureau, 1962). Kipahulu measured 12.53 inches in 12 hours, and Kaupo Ranch House, another station in southeastern Maui, registered 10.85 inches in 12 hours from 8:00 p.m. on April 15. The greatest intensity registered was 2.3 inches for a 30-minute period. Stations in other parts of the island measured little or no rain for the same period.

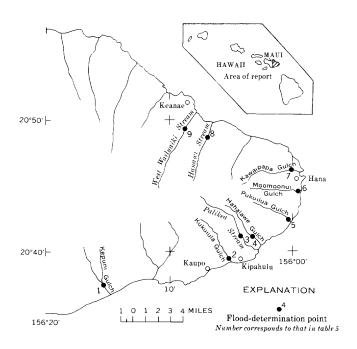


FIGURE 6.—Flood area; location of flood-determination points, floods of April 16 in eastern Maui, Hawaii.

The highway from Hana to Kipahulu was impassable due to water, mud, and debris. Sections of the road shoulders and pavement were washed out. Small landslides partially blocked the narrow paved road. Board of Water Supply pipelines were also damaged, temporarily disrupting water service in some communities. Damage in the area from Kipahulu to Kaupo was very extensive. About 6 miles of the narrow, unpaved road was closed to traffic. This highway, benched into the steep mountain slopes, is the only road serving several small villages on the eastern coast

of the island. Four timber bridges and three culverts were destroyed. Bridge approaches at four other crossings were washed out, severely damaging one bridge and causing minor damage to the others. Erosion at fords was extensive. Only one timber bridge remained undamaged from the high flows. Numerous landslides, with a total estimated volume of 5,000 cubic yards, covered the highway at several places and knocked down telephone lines. Roadbeds were also eroded by the hillside runoff. River crossings were widened considerably when bridge approaches and abutments were washed out. Several meandering stream channels were straightened; in other basins, new channels were formed in lower reaches when boulders tumbling down from upstream filled the existing streambeds. Streams in this area are normally dry or carry only trickles of water most of the year. Total damage between Kipahulu and Kaupo was about \$230,000.

The small community of Kaupo was completely isolated from Hana and Wailuku. Silt, debris, and large boulders blocked two road crossings of streams west of Kaupo.

Record-high discharges occurred at two crest-stage gages in Hana. Moomoonui Gulch (No. 6, table 5) in operation since 1963, had a peak discharge of 2,200 cfs. Kawaipapa Gulch (No. 7, table 5) with only 3 years of record, had a peak discharge of 14,000 cfs. Gaging stations at Palikea Stream (No. 3, table 5) and Hahalawe Gulch (No. 4, table 5) near Kipahulu had peak flows of 5,320 and 988 cfs, respectively, during the storm.

Table 5.-Flood stages and discharges, April 16 in eastern Maui, Hawaii

			Maximum floods						
No.	Stream and place of determination	Drainage area (sq mi)	Known before April 1968		April	Gage	Discharge		
			Period	Year	1968	height (feet)	Cfs		
1	Kepuni Gulch near Kahikinui House.	1.91	1963-68	1967	16	8.23 4.52	1,000 195		
2	Kukuiula Gulch near Kipahulu.	.76	1964-68	1965	16	5.11 16.85	1,560 2,760		
3	Palikea Stream below diversion	6.29	1927-29, 1931-68.	1965		18.3	16,100		
	dam near Kipahulu.				16	9.69	5,320		
1	Hahalawe Gulch near Kipahulu.	.43	1927–37 1939–68	1937		15.74	3,560		
5	Pukuilua Gulch	.48	1963-68	1965	16	4.88 9.3	988 788		
6	near Hana. Moomoonui Gulch at Hana.	.90	1963-68	1965	16 16	7.76 14.46 14.64	594 2,070 2,200		
7	Kawaipapa Gulch at	5.83	1965-68	1965	16	9.59 10.36	10,200 14,000		
8	Hana. Hanawi Stream near Nahiko.	3.49	1914-16, 1922-68.	1916		11.6	5,570		
					16	8.15	2,660		
9	West Wailuaiki Stream near	3.66	1914-17, 1922-68.	1923		13.50	6,960		
	Keanae.				16	8.07	2,490		

Damage in the flood area was estimated at \$293,000. The torrential rains and resulting runoff destroyed a culvert at Honomaele Gulch on Hana Highway, which was closed to traffic until a temporary bypass road was constructed. Flows over the rolling pastureland flooded the roadways in Hana and severely damaged a 200-foot section of a drainage ditch, whose lining, bedding, and sections of earth banks were washed downstream into the ocean. Road shoulders, retaining walls and pavement on a 50-foot section of the lower Hana Road near Holoinawawae Stream were also damaged. Damage was about \$35,000. The Hotel Hana Maui had about \$5,000 damage to carpets, furniture, and supplies when water rushed over the highway into the building. Roads, fences, waterlines, and feed storage areas on Hana Ranch were also damaged. Other damage in Hana consisted of toppled retaining walls and debris left at private residences.

FLOODS OF MAY IN SOUTH ARKANSAS

Heavy rains fell on south Arkansas during the period May 7-18, with the greatest amount falling on the area extending from Little Rock to the southwest corner of the State (fig. 7).

Runoff from the storm of May 9-10 was not especially heavy, but wetting of the soil at this time induced high rates of runoff from the second storm of May 13. Floods during May 9-10 were greatest in the upper reaches of the Saline River (tributary to Ouachita River), on the Little Missouri River and tributaries, and on Caney Creek (tributary to the Red River). Rainfall of up to 10 inches in 24 hours caused streams to rise rapidly, but the peaks were of short duration except those on larger and slow-moving streams.

Floods from the storm of May 13 were much more severe and widespread. The greatest flooding during the period May 16-17 was on Smackover Creek and other smaller streams in the southern part of the flood area.

Flood damage was estimated at \$18.3 million. No lives were lost. Many residential and industrial areas were flooded. Millions of acres of timberland, farmland, and pastures were inundated, eroded, and silted over. Many highway bridges were damaged.

A report by Gilstrap (1971) describes the flood in detail. The report discusses the storms, the floods, and the flood damage. Flood data in the report include peak stages and discharges at 95 sites, flood profiles of three streams, recurrence intervals of peak discharges, rainfall-runoff relations, and reservoir contents.



FIGURE 7.—Flood area, floods of May in south Arkansas.

FLOODS OF MAY 24-27 IN SOUTHERN INDIANA

Severe flooding occurred in southeastern and west-central Indiana as the result of heavy rainfall during May 22–25. Precipitation ranged from 3 to 7 inches and generally progressed from west to east across the center of Indiana. An isohyetal map of Indiana (fig. 8) was compiled from U.S. Weather Bureau data for the period May 22–25.

Maximum stages during periods of record ranging from 3 to 40 years in length were equaled or exceeded at three gaging stations (table 6). The recurrence interval of the floods equaled or exceeded 50 years at five of these gaging stations, 20 years at six stations, and 10 years at 17 stations.

Flooding in urban areas of Johnson, Shelby, and Bartholomew Counties caused widespread damage to homes and small businesses. Damage in rural areas of Jackson and Jennings Counties was estimated at \$750,000.

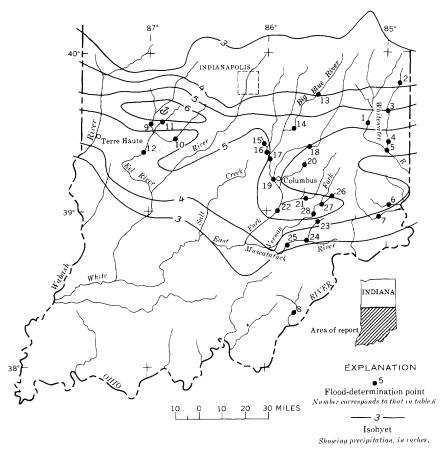


FIGURE 8.—Flood area; location of flood-determination points and isohyets for May 22-25, floods of May 24-27 in southern Indiana.

Table 6.-Flood stages and discharges, May 24-27 in southern Indiana

No.	Stream and place of determination		Maximum floods						
		Drainage area (sq mi)	Known before May 1968			Gage height	Discharge		
					May			Recur-	
			Period	Year	1968	(feet)	Cfs	rence interval (years)	
			Miami Ri	ver basin					
1	Whitewater River near Alpine	529	1929-68	1937	24	16.61 15.81	37,100 27,600	14	
2	East Fork White- water River at Richmond.	121	1913 1949–68	1913 1950 1959		15.0 12.49	(¹) 14,100	- - - 2	
3	East Fork White- water River at Abington.	198	1966-68	1967	24 24	10.06 13.54 14.05	7,500 6,300 9,980	6	
4	East Fork White- water River at Brookville.	380	1954-68	1959	24	² 16.50 17.35	36,100 31,600	18	

Table 6—Flood stages and discharges, May 24-27 in southern Indiana— Continued

						m floods			
	Stream and place	Drainage	Known before				Discl	harge	
No.	of determination	area (sq mi)	May 196 Period		May 1968	Gage height (feet)	Cfs	Recur- rence interval	
						(1000)		(years)	
		Mia	ami River basi	in—Con	itnued				
5	Whitewater River at Brookville.	1,224	1913 1916–17, 1924–68.	1913 1959		39.0 27.78	(1) 81,800		
					24	23.55	57,800	7	
			Hogan Cre	ek basin					
6	South Hogan Creek near Dillsboro.	38.2	1959 1962–68	1959 1 96 3		14.00 10.82	16,300 8,630	50	
			Laughery C	eek basi	in		****		
7	Laughery Creek near Farmers Retreat.	248	1941-68	1959	24	21.13 17.06	47,800 28,700	16	
	****		Silver Cree	k basin					
8	Silver Creek near Sellers- burg.	188	1955-68	1959	27	30.89 18.45	19,600 4,000		
			Wabash Riv	ver basir	1				
9	Big Walnut Creek near Reelsville.	326	1950-68	1957	24	18.63 17.07	27,400 15,800	17	
10	Mill Creek near Cataract.	245	1950-68	1960	24	$22.58 \\ 21.54$	11,400 10,200	8	
11	Deer Creek near Putnamville.	59.0	1955–65, 1968.	1963		12.95	10,700		
12	Eel River at	830	1875	1875	24	15.18 30.0	8,900 (1)	3 1.5	
	Bowling Green.	200	1931–68	1950	24	23.53 21.52	34,000 22,100	6	
13	Big Blue River at Carthage	184	1951–68	1963	24	14.62 9.66	12,900 4,340	4	
14	Big Blue River at Shelbyville.	421	1913 1944–68	$\frac{1913}{1963}$	24	20.2 17.70 16.10	(1) 15,800 11,500	3	
15	Youngs Creek	107	1943-68	1952	24	13.4	10,700	³ 1.1	
16	near Edinburg. Sugar Creek near	474	1943-68	1956		12.85 18.38	9,290 27,600	17	
17	Edinburg. Driftwood River	1,060	1913	1913	24	16.98 20.3	19,300		
	near Edinburg.		1941-68	1963	24	16.97 16.61	40,500 34,600	14	
18	Flatrock River at St. Paul.	303	1913 1931–68	$\frac{1913}{1949}$		20.5	(¹) 18,500		
				1963	24	$12.17 \\ 12.37$	17,600	13	
19	East Fork White River at Columbus.	1,707	1948-68	1963	25	16.23 14.99	52,300 44,700	10	
20	Clifty Creek at Hartsville.	91.4	1913 1948–68	1913 1959	24	25.1 14.29 11.79	(1) 11,300 6,840	 10	
21	Sand Creek near Brewersville.	155	1948–68	1959	24	21.70 20.08	19,900 15,200	13	
22	East Fork White River at Seymour.	2,341	1913 1928–68	1913 1949	24	21.0 19.67 18.93	120,000 78,500 60,200	13	
23	Graham Creek near Vernon.	77.2	1954-68	1960	24	21.37 19.98	18,600 16,300	46	

TABLE 6—Flood stages and discharges, May 24-27 in southern Indiana— Continued

					Maximu	m floods		
No.	G4	Destruction	17				Discharge	
	Stream and place of determination	Drainage area	Known before May 1968		34	Gage		Recur-
		(sq mi)	Period	Year	May 1968	height (feet)	Cfs	rence interval (years)
		Wal	bash River b	asin—Cor	ıtinued			
24	Muscatatuck River near Deputy.	293	1948-68	1959	24	33.1 27.05	52,200 26,300	7
25	Muscatatuck River near Austin.	359	1933-68	1959	2 5	29.20 25.68	53,900 22,200	5
26	Brush Creek near Nebraska.	11.4	1955-68	1962	24	10.90 11.40	3,460 3,440	(¹)
27	Vernon Fork near Butlerville.	85.9	1942-68	1959	24	25.41 23.20	26,200 19,500	3 Î.Î
28	Vernon Fork at Vernon.	198	1940-68	1959	24	32.83 29.46	56,800 40,600	³ 1.3

¹ Not determined.

FLOODS OF MAY 24-28 IN SOUTHERN OHIO

By E. E. WEBBER

Precipitation from 3 inches to more than 5 inches during the night of May 23-24, followed by 2 to 4 inches of additional precipitation on the night of May 26-27, produced severe flooding in southern Ohio during the period May 24-28 (fig. 9). The ground in the flood area had been saturated by antecedent precipitation of 3 to 5 inches during the period May 8-22. Throughout most of the area, the highest peak discharge occurred on May 24 or 25, followed by a lower secondary peak resulting from the storm of May 26-27. On Raccoon Creek at Adamsville (No. 5, table 7) and on Ohio Brush Creek near West Union (No. 16, table 7), the highest peak discharges occurred after the second storm period.

The highest runoff and the most severe flooding occurred in the Hocking River, Raccoon Creek, Salt Creek, Paint Creek, and Little Miami River basins. (See table 7.) The highest stage since 1907 was recorded on the Hocking River at Athens (No. 3, table 7), and the record-breaking discharge of 22,500 cfs on Raccoon Creek at Adamsville was estimated to be 1.3 times that of a 50year flood. Four hundred residents of Laurelville, in the Salt Creek basin, were forced to evacuate their homes. Near the proposed Salt Creek dam (No. 15, table 7), an unofficial stage record for the 81-year period 1888–1968 was exceeded by this flood. Extreme flooding occurred in the upper reaches of Paint Creek, where the runoff resulting from precipitation cells of 5 to 7.5

At different site and datum.

Ratio of peak discharge to 50-year flood.

inches caused considerable loss of livestock and wildlife. The midportion of Little River basin suffered the most damage in that basin; the peak of 25,400 cfs at Todd Fork near Roachester (No. 21, table 7) was estimated to be 1.4 times that of the 50-year flood.

Locations of the 24 flood-determination points, and isohyets for May 23–24, 1968, are shown in figure 9. A summary of peak stages, discharges, and related data at the 24 sites is listed in table 7. Composite flood-frequency curves applicable to Ohio (Cross and Webber, 1959, and Speer and Gamble, 1965) were used to estimate the recurrence intervals shown in table 7. The greatest urban damage was at Laurelville and Athens, and throughout the storm area many roads and bridges were destroyed. Two lives were lost. Total damage in the 31 counties affected has been estimated by the U.S. Weather Bureau at \$9 million.

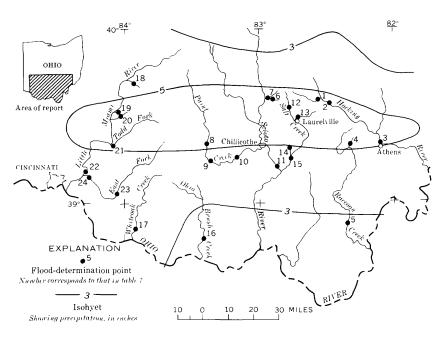


FIGURE 9.—Flood area; location of flood-determination points and isohyets for May 23-24, floods of May 24-28 in southern Ohio.

Table 7.—Flood stages and discharges, May 24-28 in southern Ohio

16,000 11,000 36,000 16,600 50,000 32,400	Recurrence interva (years)
16,000 11,000 36,000 16,600 50,000 32,400	rence interva (years)
11,000 36,000 16,600 50,000 32,400	1 1.1 15
11,000 36,000 16,600 50,000 32,400	15
11,000 36,000 16,600 50,000 32,400	15
16,600 50,000 32,400 3,770	
32,400	27
3,770 2,440	
3,770 2,440	
2,770	5
16,000	
22,500	1 1.3
2,100 3,820	1 2.0
4,390	(²)
16,000	
	(2)
56,900	
	10
177,000	
2,780 5,360	¹ 1.9
20,800	(²)
2 374 5 957	
31,600 59,000	1 2.7
59,200	
26,300	5
22,400	
12,400	4
855 1,8 2 0	1 1.2
	(²)
	(²)
	3,820 4,390 16,000 3,21,700 45,000 32,800 56,900 40,200 (2) 177,000 89,500 2,780 5,360 20,800 20,800 59,000 7 26,300 7 26,300 7 21,400 7 3 12,400 85,500 22,900 7 26,300 7 22,400 7 3 12,400 7 3 12,400

			Maximum floods							
	Stream and place	Drainage	Known b	efore			Discharge			
No.	of determination	area	May 19		May 1968	Gage	Cfs	Recur- rence interval (years)		
		(sq mi)	Period	Year		height (feet)				
		Little M	iami River ba	sin—Con	tinued					
21	Todd Fork near Roachester.	219	1953-68	1959	24	19.50 19.49	25,500 25,400	11.4		
22	Little Miami River at Milford.	1,203	1913-68 1915-17, 1925-36, 1939-68.	1913 1959		25.5 22.3	(2) 84,100			
					24	17.75	52,200	18		
23	East Fork Little Miami River at	237	1949-53, 1960-68.	1964		15.23	19,800			
	Williamsburg.				24	13.43	15,400	5		
24	East Fork Little Miami River at	476	1915–20, 1925–68.	1964		23.84	42,400			
	Perintown.				24	21.87	31,500	7		

Table 7.—Flood stages and discharges, May 24-28 in southern Ohio— Continued

FLOODS OF MAY 29-30 IN SOUTHEASTERN NEW YORK

By F. LUMAN ROBISON

The most serious flood event of the year in New York occurred in the southeastern part of the State at the end of May.

Very heavy rains of 3.0 to 6.3 inches centered in the extreme southeastern counties caused severe flooding of buildings, streets, highways, crops, and low-lying areas in Ulster, Orange, Westchester, Rockland, and Nassau Counties and in the boroughs of New York City. Thousands of acres of valuable truck-garden crops were inundated in Orange County.

In most areas the heavy rain began about 10 p.m. on May 28 and ended before noon on the next day. The maximum official U.S. Weather Bureau measurement of the storm was 5.54 inches at Dobbs Ferry. At Central Park in New York City a reading of 4.88 inches broke a 60-year record for 24-hour rainfall. At Suffern and at Greenwood Lake, unofficial measurements of 6.30 inches and 6.07 inches, respectively, were reported. The extent and distribution of rainfall is shown in figure 10. This storm extended into the area described in the following section on the floods of May 29–31 in northeastern New Jersey.

The intensity of the storm caused streams to rise rapidly. The peak discharge of the Bronx River at Bronxville (No. 5, table 8) was the greatest since establishment of the gaging station in 1944. The peak discharge of the Mamaroneck River at Mamaroneck (No. 3, table 8) was the second highest in the 24-

¹ Ratio of peak discharge to 50-year flood.

² Undetermined.

³ Site and datum then in use.

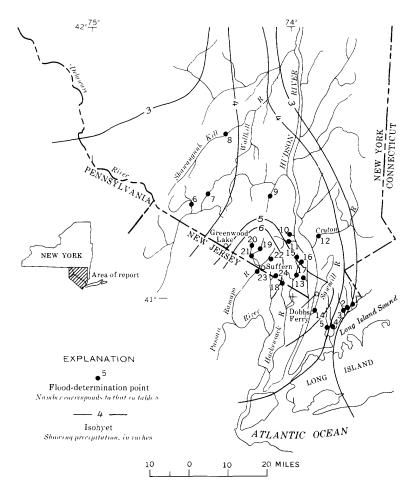


FIGURE 10.—Flood area; location of flood-determination points and isohyets for May 28-29, floods of May 29-30 in southeastern New York.

year record. The only long-term record (50 years) in the Ramapo River basin is near Mahwah, N.J. (No. 23, table 8) about 1 mile below the New York-New Jersey State line, where the peak discharge on May 29 was exceeded only by those of the floods of October 1903 and October 1955. On several small streams in Rockland County, where gages were installed in 1959 and 1960, the discharge from this storm exceeded any previously recorded.

Table 8 contains peak discharges for many streams in the flood area, with corresponding maximum peaks of record prior to May 29 for comparison. Data for some of the May flood peaks were obtained from records at U.S. Geological Survey gaging sta-

tions while others are the results of indirect measurements of peak discharges made after the floods by Survey engineers. Sites where flood discharges were determined are shown in figure 10.

Flood-frequency relationships are not well defined for the areas affected by these floods. Manmade influences due to urbanization affect the peak discharge for many streams. Gaging-station records for small areas are mostly of short duration; therefore, frequency relations for these small areas are uncertain. On the basis of the regional flood-frequency relationships, the peak discharge of 8,600 cfs on May 29 had a recurrence interval of about 25 years for the long-term station on Ramapo River near Mahwah, N.J., which has a comparatively large drainage area of 118 square miles. A study of the peak discharges listed in table 8 indicates that several were at least equal to that of a 25-year event.

The storm indirectly caused the death of a man from Blauvelt. When the eastbound lane of State Highway 59 was closed by flooding, he was riding his motorcycle east in the westbound lane and collided with a car.

Many streets and cellars were flooded in the villages of Orange and Rockland Counties and several families were forced to leave their homes. The village of Suffern was nearly isolated for a time when both east and west approaches of State Highway 59 were blocked by high water. An indication of the amount of flood damage suffered was the declaration by the U.S. Small Business Administration that Orange and Rockland Counties were disaster areas.

Table 8.—Flood stages and discharges, May 29-30 in southeastern New York

	Stream and place of determination		Maximum floods								
No.		Drainage area	Known be May 19	35.	Gage height	D.					
		(sq mi)	Period	Year	May 1968	(feet)		charge cfs)			
			Blind Brook bas	in							
1	Blind Brook at Rye.	9.20	1944-68	1955	29	9.6 6.6		1,360 896			
		Bea	ver Swamp Brool	k basin							
2	Beaver Swamp Brook near Harrison.	4.71	1944-68	1962	29	3. 1.	09 81	167 86			
		Ma	maroneck River	basin							
3	Mamaroneck River at Mamaroneck.	23.4	1938 1944-52, 1955-68.	1938 1944 1955		¹ 11.1 ³ 7.8	32	(²) 1,940			
					29	5.	69	1,840			

See footnotes at end of tables.

Table 8.—Flood stages and discharges, May 29-30 in southeastern New York—Continued

No45	Stream and place of determination Hutchinson River at Pelham. Bronx River at	Drainage area (sq mi) H	Known be May 19 Period outchinson River		May 1968	Gage height	Dis-
	at Pelham. Bronx River at			Year	1069		
	at Pelham. Bronx River at		utchinson River		1900	(feet)	(cfs)
	at Pelham. Bronx River at	5.76		basin			
5			1944-68	1962	29	4.98 4.02	390 166
5			Bronx River ba	sin			
	Bronxville.	2.65	1944-68	1955	29	6.65 6.35	922 1,450
			Hudson River b	asin			
6	Rutgers Creek at Gardnerville.	59.7	1944-68	1955	30	12.38 5.84	8,490 1,580
7	Wallkill River at Pellets Island Mountain.	885	1920-68	1936	30	$\frac{26.0}{13.7}$	12,400 3,750
8	Shawangunk Kill at Pine Bush.	102	19 25-3 2, 19 52, 19 55 -68.	1955		12.5	9,700
9	Woodhuur Cuole	11.9	1055 69	1967	2 9	10.03	6,840
y	Woodbury Creek near Highland	11.2	1955-68	1967	22	3.04	150
10	Mills.	150	1050 40	1001	29	4.97	917 830
10	Cedar Pond Creek at Stony Point.	17.3	1959-68	1961	29	4.95 8.87	1,920
11	South Branch Minisceongo Creek at	5.83	1960-68	1960	29	3.13 3.84	120 230
	Letchworth Village						
12	Croton River at New Croton Dam near Croton-on- Hudson.	378	1934-68	1955	30	18.44 6.69	45,400 4,740
13	Sparkill Creek at Sparkill.	11.1	1960-68	1962	2 9	3.86 5.34	438 830
14	Sawmill River at Yonkers.	25.6	1944-68	1955	30	5.34 4.65	890 481
		H	ackensack River	basin			
15	Hackensack River at Brookside Park.	13.2	1959–63, 1966–68.	1960		6.67	1,010
16	East Branch Hackensack river at Congers.	6.86	1959-68	1960	29 29	7.40 9.12 9.29	1,350 110 125
17	Hackensack River at West Nyack.	29.4	1959-68	1960	29	6. 23 6.84	65 4 797
18	Pascack Brook tributary at Spring Valley.	4.58	196068	1960	29	4.46 5.16	378 580
			Passaic River ba	sin			
19	Ramapo River at Sloatsburg.	60.9	1955 1959–68	1955 1960	29	12.2 9.52 11.4	6,240 2,270 4,600
20	Stony Brook at Sloatsburg.	18.2	1960-68	1960	29	3.14 5.23	1,060 1,650
21	Ramapo River tributary at Sloatsburg.	5.35	1959-68	1960	29	7.05 10.13	180 540
22	Mahwah River near Suffern.	12.3	1958-68	1960	29	5.76 7.79	462 1,650

See footnotes at end of table.

				Maxi	imum floods	ı	
	Stream and place of determination	Drainage area (sq mi)	Known before May 1968		7.5	Gage	Dis-
No.			Period	Year	Мау 1968	height (feet)	charge (cfs)
		Passai	c River basin—	-Continued			
23	Ramapo River near Mahwah,	118	1903-06, 1923-68.	1903		⁸ 11.0	12,400
	N. J.				29	11.87	8,600
24	Pine Brook near Spring Valley.	2.2 8	1960-68	1962	29	2.89 3.42	120 170

Table 8.—Flood stages and discharges, May 29-30 in southeastern New York—Continued

FLOODS OF MAY 29-31 IN NORTHEASTERN NEW JERSEY

Extremely heavy rains over the Passaic River basin and adjoining areas caused major flooding in northeastern New Jersey (fig. 11). The rainfall occurred on May 28-30, with much of the precipitation measured on May 29. The heaviest rains evidently occurred on the ridge around Chatham, where a total of 7.07 inches was reported. The second highest rainfall of 6.67 inches was reported at Charlottesburg. This storm extended into the area described in the preceding section on the floods of May 29-30 in southeastern New York. Peak discharges at gaging stations in the Hackensack, Saddle, and Rahway River basins and on the Passaic River near Chatham were equal to or greater than those of a 50-year flood. The peak discharge of 2,560 cfs at Chatham (No. 5, table 9) was the highest since January 1905. Maximum discharges for the period of record (since 1922 at some stations) occurred at the gaging stations in the Hackensack and Rahway River basins. (See table 9.)

Northeastern New Jersey was declared a disaster area. More than 2,000 families were evacuated from flooded areas. The Small Business Administration estimated the flood damage at \$133 million. Seven deaths occurred due to drowning.

¹ Affected by hurricane wave.

² Unknown. At datum 0.41 foot higher than present datum.
Affected by backwater from ice.
At site 250 feet downstream at different datum.

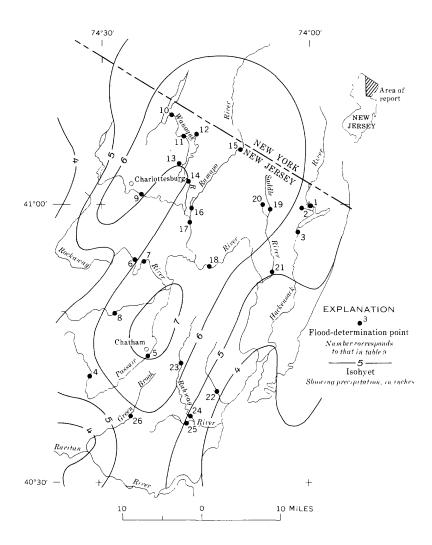


FIGURE 11.—Flood area; location of flood determination points and isohyets for May 28-30, floods of May 29-31 in northeastern New Jersey.

 $\begin{array}{c} {\bf TABLE~9.--Flood~stages~and~discharges,~May~29\text{--}31~in~northeastern} \\ {\color{blue}New~Jersey} \end{array}$

					Maximu	n floods		
	Stream and place	Drainage	Known be	fore			Disc	charge
No.	of determination	area (sq mi)	May 19 Period	Year	May 1968	Gage height (feet)	Cfs	Recur- rence interva (years)
			Hackensack R	iver basir	n			
1	Hackensack River at Rivervale.	58.0	1942-68	1955	29	6.03 6.23	1,450 1,500	50
2	Pascack Brook at Westwood.	29.6	1935-68	1945	29	6.76 6.61	1,610 1,640	30
3	Hackensack River at New Milford.	113	1922-68	1951	30	6.14 6.60	3,660 4,040	50
			Passaic Rive	er basin				
4	Passaic River near Milling-	55.4	1904-06, 1922-68.	1905		1 7.8	2,000	
5	ton. Passaic River near Chatham.	100	1903-11, 1938-68.	1905	29	8.04 ² 8.3	1,070 3,000	8
	*				29	7.90	2,560	3 1.07
6	Rockaway River above reservoir at Boonton.	116	1938-68	1952	29	6.62 6.22	3,250 3,020	10
7	Rockaway River below reservoir	119	1903, 1906–68.	1903			4 7,560	
8	at Boonton. Whippany River	29.4	19 22 –68	1928	29	8.20 7.3	3,450 2,000	(⁵)
9	at Morristown. Pequannock River at Macopin In-	63.7	18981968	1903	29 29	5.16 3.85 2.15	956 6,100 2,510	3 (⁵)
10	take Dam. Wanaque River at Awosting.	27.1	1919–68	1955	29	5.85 4.99	1,300 875	8 .
11	Wanaque River at Monks.	40.4	1935–68	1955	29	4.15 3.97	3,640 3,330	30
12	Ringwood Creek near Wanaque.	19.1	1935-68	1951	29	3.74 3.72	1,150 1,020	5
13	West Brook near Wanaque.	11.8	1935-68	1951	29	6.6 4.35	1,900 818	5
14	Wanaque River at Wanaque.	9.04	1913–15, 1919–68.	1951		9.12	8,470	
	D	440			29	8.42	5,920	(⁵)
15	Ramapo River near Mahwah.	118	1903-06, 1923-68.	1903	29	6 11.0 11.87	12,400 7,740	12
16	Ramapo River at	160	1922-68	1936			12,300	
	Pompton			1955		4.40		7.0
17	Lakes. Pompton River at Pompton	355	1903-04, 1940-68.	1903	2 9	3.82 7 14.3	9,530 2 8, 34 0	12
18	Plains. Passaic River at	762	1898–1968	1903	30	22.18	13,100 4 28,000	15
19	Little Falls. Saddle River at	21.6	1945	1945	31	10.73	13,500 6,400	10
	Ridgewood.		1955-68	1955	29	8.88 9.80	1,510 2,060	³ 1.05
20	Hohokus Brook at Hohokus.	16.4	1954-68	1955	29	$\frac{4.50}{4.31}$	2,350 2,120	24
21	Saddle River at Lodi.	54.6	1924–68	1945	29	10.00 9.53	3,500 3,330	⁸ 1.06
			Elizabeth Ri	ver basin				
22	Elizabeth River at Elizabeth.	20.2	1922-68	1938 1966		8 16.19	2,720	
				1900	29	15.09	2,450	13

See footnotes at end of table.

Table 9.—Flood stages and discharges, May 29-31 in north	eastern
$New\ Jersey$ —Continued	

	C4 1 1.		Maximum floods							
		7 5. 1	Known l	ofowo			Discharge			
No.	Stream and place of determination	Drainage area	May 1		May 1968	Gage height (feet)		Recur-		
		(sq mi)	Period	Year			Cfs	rence interval (years)		
			Rahway Ri	ver basin						
23	Rahway River near Springfield.	25.5	1938-68	1938	29	7.51 8.50	1,940 2,520	³ 1.37		
24	Rahway River at Rahway.	40.9	1922-68	1938	29	6.35 6.67	3,140 3,530	³ 1.04		
25	Robinsons Branch Rahway River at Rahway.	21.6	1940–68	1953	29	5.36 6.00	1,490 2,550	³ 1.25		
			Raritan Ri	ver basin		_				
26	Green Brook at Plainfield.	9.75	1938-68	1938	29	5.82	9 2,890 9 2,800	(5)		

At site 0.75 mile downstream at different datum.

At bridge 150 feet upstream at different datum.

Ratio of peak discharge to 50-year flood.

Daily discharge.

Not determined.

8 Affected by debris.
9 Part of flow at extreme high stages bypasses the station; amount undetermined.

FLOODS OF JUNE AND AUGUST IN WYOMING

By D. J. O'CONNELL

Numerous floods occurred throughout Wyoming in June and August. A combination of snowmelt and rainfall produced major floods in June on streams which flow easterly from the Bighorn Mountains in the Yellowstone River basin in north-central Wyoming and on streams which flow northeasterly from the Uinta Mountains in the Green River basin in southwestern Wyoming. Also, locally heavy thunderstorms produced significant floods in the north-central and southwestern parts of the State in June and August. Stages and discharges for these floods are listed by basin in downstream order in table 10.

NORTH-CENTRAL WYOMING

Floods in June in north-central Wyoming were the result of a combination of rainfall and snowmelt runoff on streams flowing easterly from the Bighorn Mountains and were the result of general rains and locally intense thunderstorms on streams flowing westerly from Big Horn Mountains (fig. 12).

The snowpack on the Bighorn Mountains was decreasing rapidly when the rains began on June 4. The easterly flowing streams were not vet in their recession phase, but the westerly

At site 250 feet downstream at different datum.
At site, 1,600 feet upstream at different datum.

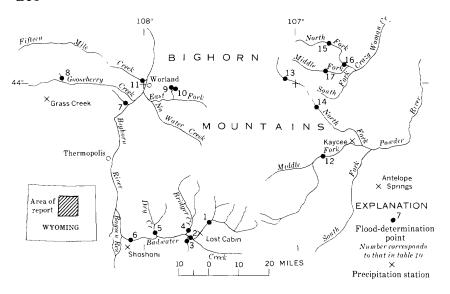


FIGURE 12.—Flood area; location of flood-determination points and precipitation stations, floods of June and August in north-central Wyoming.

flowing streams were well down in their recession phase. The amounts of rainfall recorded at nearby U.S. Weather Bureau stations, in inches, are tabulated below:

Jun	e	Antelope Springs	Каусее	Lost Cabin	Shoshoni
4		. 0	0.04	0.06	0.02
5		.45	.58	.23	.25
6	~	1.01	1.14	1.15	.50
7	-	.17	.03	Trace	.07
8		.10	.20	.98	0
9	-	.25	.64	.87	.23
	Total	1.98	2.63	3.29	1.07

The rains caused rapid melting of the remaining snowpack and the consequent runoff caused several peak discharges of record at gaging stations on streams flowing in an easterly direction (Nos. 12–17, table 10). The rain received by June 6 increased the soil moisture on drainages for streams flowing in a westerly direction and produced minor peaks in Badwater Creek. Additional rain on June 8 and 9 and favorable antecedent runoff conditions caused peak discharges of record at six gaging sites on Badwater Creek and its tributaries (Nos. 1–6, table 10).

Scouring of the banks along Badwater Creek during the flood endangered the tracks of the Chicago, Burlington & Quincy Railroad near the town of Bonneville and caused minor concern but no delays in rail traffic. Other damage from floodwater was limited to rural areas and was minor.

A general rainstorm in the Thermopolis-Worland area on August 23 seemingly reached its greatest intensity in the headwaters of Little Gooseberry Creek. The sites where peak discharges were determined are indicated in figure 12 (Nos. 7–11), and discharges are shown in table 10. The U.S. Weather Bureau stations at Grass Creek and at Worland Airport measured 1.32 and 1.12 inches of precipitation, respectively, on August 23. The peak discharge on Little Gooseberry Creek (No. 7, table 10) was 2.2 times that of the 50-year flood. The Chicago, Burlington & Quincy Railroad bridge over Little Gooseberry Creek was damaged and rail traffic was disrupted.

SOUTHWESTERN WYOMING

Floods in June in southwestern Wyoming resulted from a combination of rainfall and snowmelt runoff on streams which flow northeasterly from the Uinta Mountains (fig. 13). The Uinta Mountains were overlain with a heavy snowpack when the rains began on June 4. The amounts of rainfall recorded at nearby U.S. Weather Bureau stations, in inches, are tabulated below:

Date	Mountain View	Church Buttes	Green River	Manils
June				
4	. 0	0	Trace	0.10
5	1.11	.81	0.62	.88
6	.18	.39	.22	.22
7	. 0	Trace	.02	.14
8	.09	0	.14	.07
9	0	.20	.03	.05
10	.27	.15	Trace	.04
Total	1.65	1.55	1.03	1.50
Aug.				
9	.43	.41	.02	.67
10	.24	.36	.54	0
11	.03	Trace	.15	.15
Total	.70	.77	.71	.82

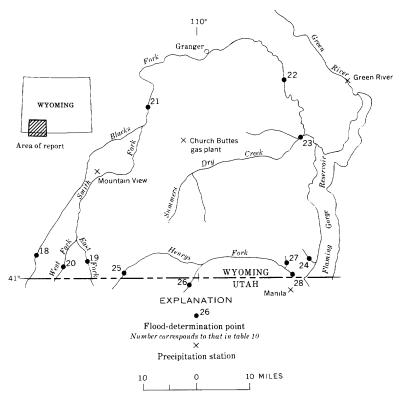


FIGURE 13.—Flood area; location of flood-determination points and precipitation stations, floods of June and August in southwestern Wyoming.

Rain and snowmelt runoff combined to produce peaks that were greater than that of the 50-year flood at two of the streamgaging stations (Nos. 20, 25; table 10).

Floodwaters caused minor damage to property in Mountain View and in rural areas on the flood plains. A bridge was damaged at Mountain View.

A rainstorm over southwestern Wyoming produced significant flood peaks at three partial record stations (Nos. 23, 24, 27, table 10) and at one recording station (No. 28, table 10) on August 11. Maximum peaks of record were recorded at two of the partial record stations (Nos. 23, 27; table 10). No damage was reported from this flood.

Table 10 .- Flood stages and discharges, June and August, in Wyoming

					Maximum	floods		
No.	Stream and place of determination	Drainage area	Known June		June,	Gage	Disc	harge Recur-
110.	or determination	(sq mi)	Period	Year	August 1968	height (feet)	Cfs	rence interva (years)
			Yellowston	e River ba	ısin			
1	Badwater Creek at Lybyer Ranch near	131	1949-68	19 52 1967	June 9	¹ 8.88 5.16 6.18	708 1,060	4
2	Lost Cabin. Badwater Creek	415	1966-68	1967	<u>.</u> <u>:</u> -	6.55	2,260	22
3	at Lysite. Badwater Creek tributary near	5.86	1965–68	1967	June 8 June 8	7.92 4.10 5.69	6,240 512 1,460	20 2 1.8
4	Lysite. Bridger Creek	182	1966-68	1967	Tuno 0	6.80	727	- <u>-</u>
5	near Lysite. Dry Creek near Bonneville.	52.6	1966-68	1967	June 9 June 9	7.52 12.41 3 9.64	880 1,650 584	3
6	Badwater Creek at Bonneville.	808	1923 1947-68	19 23 1956		(4) 6.60	18,600 7,260	
7	Little Gooseberry Creek near Worland.	30.2			June 9 Aug. 23	7.64	8,490 1,980	16 ² 2.2
8	Murphy Draw near Grass Creek.	2.32	1965-68	1967	Aug. 23	6.05 4.90	583 244	(⁴)
9	North Prong East Fork No Water Creek near Worland.	3.8	1964–68	1967	Aug. 23	5.46 4.96	391 242	(4)
10	North Prong East Fork No Water Creek tributary near Worland.	2.1	1965–68	1967	Aug. 23	4.43 4.00	(4) 114	(⁴)
11	Fifteen Mile Creek near Worland.	518	1951-68	1952	Aug. 23	⁵ 5 .77 5.11	3,300 1,540	3
12	Middle Fork Powder River above Kaycee.	450	1949-68	1963	June 8	8.45 8.74	1,610 1,750	6
13	North Fork Powder River near Hazelton.	24.5	1947-68	1953	June 8	⁵ 4.34 4.95	886 820	35
14	North Fork Powder River near Mayo- worth.	106	1941-68	1941	June 9	⁵ 7.64 6.90	1,270 952	12
15	North Fork Crazy Woman Creek below Spring Draw near Buffalo.	51.7	1950-68	1966	June 9	5.99 6.03	1,150 1,290	10
16	North Fork Crazy Woman Creek near Greub.	174	1950-68	1962	June 8	9.05 11.29	1,050 1,640	6
17	Middle Fork Crazy Woman Creek near Greub.	82.7	1942-68	1947	June 9	⁵ 5.77 6.00	4,520 1,470	8
			Green I	River basin	1			
18	Blacks Fork near Milburne.	156	1940-68	1957 1965	June 20	6.76 6.07	2,530 1,940	4
19	East Fork of Smith Fork near Robertson.	53.0	1940-68	1965	June 6	6.75 6.63	1,450 1,280	21

See footnotes at end of table.

Table 10.—Flood stages and discharges, June and August, in Wyoming— Continued

	Stream and place				Maximum	n floods		
		D	V h			G	Disc	harge
No.	of determination	Drainage area (sq mi)	Known b June 1		June, August	Gage height (feet)		Recur- rence
		(8q III)	Period	Year	1968	(reet)	Cfs	interval (years)
		Gre	en River bas	in—Conti	inued			
20	West Fork of Smith Fork near Robert- son.	37.2	1940-68	1965	June 5	6 3.20 4.15	2,100 1,060	2 1.02
21	Blacks Fork near Lyman.	821	1938-57, 1962-68.	1965	June 7	12.68 10.47	7,960 4,000	(⁴)
22	Blacks Fork near Little America.	3,100	1962–68	1965	June 8	10.90 8.43	9,980 4,380	(⁴)
23	Summers Dry Creek near Green River.	423	1965-68	1965	Aug. 11	9.63 10.73	2,700 3,830	(4)
24	Green River tributary No. 2 near Burntfork.	13.0	1959, 1961–68.	1959	Aug. 11	20.52 7.97	3,360 490	(4)
25	Henrys Fork near Lonetree.	56	1943-68	1965	June 6	5.79 5.24	2,010 1,490	² 1.3
26	Burnt Fork near Burntfork.	52. 8	1943–68	1965	June 5	(⁴) 4.35	3,200 782	13
27	Henrys Fork tributary near Manila, Utah.	3.15	1965-68	1965	Aug. 11	5.81 8.89	(4) 588	(4)
28	Henrys Fork at Linwood, Utah. ⁷	520	1929–68 1929–58, 1960–68.	1959 1936	June 6 Aug. 11	⁵ 9.42 ⁵ 7.19 6.51 6.67	(4) 6,750 1,960 2,040	2

¹ Affected by backwater from ice.

2 Ratio of peak discharge to 50-year flood.
3 Affected by backwater from debris.
4 Not determined.

⁵ At different site and datum. ⁶ Datum, 2.00 feet higher.

⁷ In Sweetwater County, Wyo., 0.8 miles north of Utah-Wyoming state line, 1.5 miles northwest of Linwood, Utah.

FLOODS OF JUNE 23-28 IN CENTRAL AND EASTERN TEXAS

By L. G. STEARNS

On the afternoon of June 23, tropical storm Candy moved inland over the middle Texas coast. The storm weakened slowly as it moved north toward Dallas-Fort Worth metropolitan area. As a result of this storm, rainfall totals of 3 to 4 inches were common throughout central and eastern Texas, with numerous U.S. Weather Bureau stations recording 5 or more inches.

The heavy rains associated with tropical storm Candy followed 8 to 10 consecutive days of rain, thereby causing serious flooding along the middle and upper coastal regions (fig. 14).

Flooding in the San Jacinto River basin resulted in Lake Houston cresting at 47.52 feet on June 26, with flooding continuing through the end of June. The peak stage recorded at Lake Houston, 3 feet over the top of the spillway, was the second high-

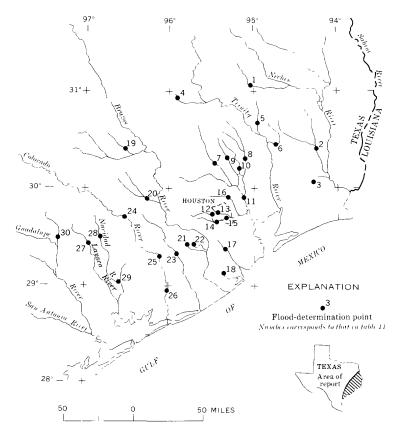


FIGURE 14.—Flood area; location of flood-determination points, floods of June 23-28 in central and eastern Texas.

est stage of record and was surpassed only by the flood of September 12, 1961, which was a result of Hurricane Carla. No historical peak discharges were exceeded during this period; however, maximum peak discharges for the 1968 water year did occur at 30 stream-gaging stations in central and eastern Texas during the period June 23–28 (table 11).

No deaths or injuries resulted from this storm; however, estimates placed crop losses at \$2,105,000 and other property losses at \$625,000.

Table 11.—Flood stages and discharges, June 23-28 in central and eastern Texas

					Maximu	m floods		
	Stream and place	Drainage	Known be	fava			Disch	arge
No.	of determination	area (sq mi)	June 1968 Period Year		June	Gage height		Recur- rence
			Period	Year	1968	(feet)	Cfs	interva (years)
			Neches Rive	r basin				
1	Piney Creek near Groveton.	79.0	1921-68 1962-68	1942 1966	25	17 13.28 14.53	(1) 1,660 4,280	4
2	Village Creek near Kountze.	860	1884-1968 1924- 27 , 1939-68.	1915 1940		² 34 27.6	(1) 67,200	
3	Pine Island Bayou near Sour Lake.	336			26 28	* 18.32 27.28	9,850 3, 300	2 2
			Trinity Rive	er hasin				
	Comerc Correla	110					(1)	
•	Caney Creek near Madison- ville.	112	1900-68 1963-68	1929 1966	24	22 17.37 17.32	(1) 15,200 11,800	18
5	Long King Creek at Livingston.	141	1870-1968 1963-68	1929 1966	24	41 18.17 17.40	(1) 6,500 6,700	5
6	Menard Creek near Rye.	152	1961-68 1966-68	1961 1966	26	34 4 23.89 20.74	(1) 5,090 1,810	2
			San Jacinto R	iver basi	n			
7	West Fork San Jacinto River	809	1913-68	1940	25	30.85 22.90	110,000 24,300	4
8	near Conroe. East Fork San Jacinto River near Cleveland.	325	1900-68	1940	24	⁵ 20.37 19.57	59,000 11,900	3
9	Caney Creek near Splen- dora	105	1885-1968 1944-68	1940 1945	23	27 23.19 17.97	(¹) 14,900 3,670	3
10	Peach Creek at Splendora.	117	1895–1968	1949	25	22.73 17.00	28,500 4,200	-3
11	Lake Houston near Sheldon.	2,828	1954-68	1961	26	47.87 47.52	6 205,000 6 187,100	
12	Keegans Bayou at Roark Road near Houston.	9.28	1965–68	1966	23	67.64 67.89	588 352	
13	Brays Bayou at Houston.	88.4	1911–68	1919 1960	23	⁷ 56.0 49.72 40.48	(1) 12,600 12,000	28
14	Sims Bayou at Hiram Clarke Street, Houston.	20.2	1965-68	1966	23	51.08 52.35	2,280 2,200	12
15	Sims Bayou at Houston.	64.0	1953-68	1960	24	29.76 24.80	8,030 4,680	12
16	Greens Bayou near Houston.	72.7	1953-68	1954 1961	24	65.75 62.82	7,000 2,580	 2
			Chocolate Ba					
17	Chocolate Bayou near Alvin.	87.7	1947-68	1949	24	8 21.80 20.34	7,400 4,160	2
	near Aivill.		Oyster Cree		44	40.04	4,100	
10	Ovator C	911				90.0	(1)	
18	Oyster Creek near Angleton.	211	1900-68 1945-68	1913 1957	26	32.2 31.45 27.78	9 10,600 3,080	2

See footnotes at end of table.

Table 11.—Flood stages and discharges, June 23-28 in central and eastern Texas—Continued

					Maximun	floods		
	Stream and place	Drainage	Known befo	re			Disch	arge
No.	of determination	area (sq mi)	June 1968		June 1968	Gage height	G¢.	Recur- rence
			reriod	Year	1968	(feet)	Cfs	interval (years)
			Brazos River	basin				
9	Davidson Creek near Lyons.	195	1902–68 1962–68	1947 1962	24	17 16.08 18.67	(¹) 6,000 23,200	34
20	Mill Creek near Bellville.	377	1899–1968 1963–68	1940 1965	25	22.8 15.20 15.98	(1) 26,200 34,000	34
21	Big Creek near Needville.	42.3	1913-68 1947-68	1945 1960	23 or 24	8 24.4 8 23.81 20.71	(1) 10,400 3,400	1 <u>4</u>
22	Dry Creek near Rosenberg.	8.53	1932-68	1959	24	12.66 10.30	2,410 860	(¹)
			San Bernard Ri	ver bas	in			
23	San Bernard River near Boling.	727	1900-68 1954-68	1913 1960	27	43.5 42.41 36.75	(¹) 21,200 13,200	2
			Colorado Rive	r basin				
24	Colorado River at Columbus.	10 41,070	1852-1968	1869 1913		41.6 41.6	(1) (1)	
			1916–68	1935	25	$\frac{38.5}{24.16}$	190,000 49,800	6
25	Colorado River at Wharton.	¹⁰ 41,380	1869-1968 1919-25,	1869 1913 1935		38.9 38.9 38.2	(1) (1) 159,000	
			1929, 1935, 1938–68.		26	28.06	55,400	8
26	Colorado River near Bay City.	¹⁰ 41,650	1869-1968 1940,	1869 1913 1960		56.1 56.1 48.2	(1) (1) 84,100	-
			1948-68.		26	37.49	49,500	6
			Lavaca River	basin				
27	Lavaca River at Hallettsville.	101	1840-68	1940	24	40.60 24.44	93,100 6,590	3
2 8	Navidad River near	333	1860-1968 1962-68	1940 1967		40 30.34	(¹) 19,500	
29	Hallettsville. Navidad River near Ganado.	1,116	1876–1968	1936	24 26	28.28 39.8 33.15	12,800 94,000 32,200	3 6
			Guadalupe Riv	er basi	in			
30	Peach Creek below Dilworth.	462	1840-1968 1960-68	1940 1960	25	35.3 31.28 28.86	(¹) 23,800 10,400	

¹ Not determined.

 $^{^2}$ At site 2,000 feet downstream at same datum.

³ At different site and datum.

⁴ Result of dam failure.

⁵ At site 1,800 feet upstream and datum 5,000 feet higher.

⁶ Contents in acre-feet.

⁷ At former site before channel rectification.

⁸ Before channel rectification.

⁹ Includes overflow from Brazos River.

 $^{^{10}}$ Of which 11,900 sq mi is probably noncontributing.

FLOODS OF JUNE 24 IN CENTRAL NEBRASKA

By H. D. BRICE

Widespread rain fell on June 23–24 in central Nebraska. The larger amounts recorded were 5 to 6 inches in the upper reach of Wood River and 7 to 10 inches in the lower reach of South Loup River (fig. 15, table 12). A rancher living 1 mile east of Rockville reported that he measured 3.6 inches of rain in 1 hour, a point-rainfall rate having a recurrence interval of about 100 years (U.S. Weather Bureau, 1961). Hail and high-velocity wind accompaning the rain caused considerable damage.

Runoff from the storm caused Wood River to overflow in the vicinity of Lomax and South Loup River to overflow from above Ravenna to below St. Michael. The peak discharge near Lomax, 1,270 cfs, was the third largest in the 17 years of record and its recurrence interval is estimated to be 40 years (table 13). The peak discharges at Ravenna and St. Michael were greater than those for a 50-year flood.

Because the approaches to bridges were washed out and electric power and telephone service were cut off, Ravenna was almost completely isolated for a time. Wind, hail, and flood damages from the storm were estimated at \$1,260,000.

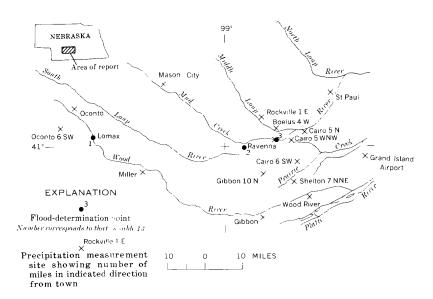


FIGURE 15.—Flood area; location of flood-determination points and rainfall data sites, floods of June 24 in central Nebraska.

Table 12.—Daily rainfall, in inches, associated with floods of June 24 in central Nebraska

	m:		- Total			
Stations	Time Observation	23	24	1 25	- Total	
Boelus 4 W		0	9.0	0	9.0	
Cairo 5 WNW		0	7.0	0	7.0	
Cairo 5 N		Ō	7.2	2.0	9.2	
Cairo 6 SW		0	5.5	1.5	7.0	
Gibbon	2400	0	(²)	7.0	7.0	
Gibbon 10 N		0	9.1	1.2	10.3	
Grand Island Airport	2400	3.09	1.79	.63	5.51	
Mason City	0800	0	2.29	1.90	4.19	
Miller	1800	Ó	5.07	.76	5.83	
Oconto		Ō	2.8	2.2	5.0	
Oconto 6 SW	1800	Ō	3.82	1.81	5.63	
Ravenna	0700	Ō	7.18	1.45	8.63	
Rockville 1 E		Ō	3 5.8	0	5.8	
St. Paul		1.74	1.14	.27	3.15	
Shelton 7 NNE		0	6.0	0.7	6.0	
Wood River		ŏ	6.0	.9	6.9	

¹ Readings for this day include rain that fell during part of the preceding day.

Table 13.—Flood stages and discharges, June 24, Platte River basin, central Nebraska

			Maximum floods							
							Discl	Discharge		
No.	Stream and place of determination	Drainage area	Known before June 1968		.	Gage		Recur-		
		(sq mi)	Period	Year	June 1968	height (feet)	Cfs	rence interval (years)		
1	Wood River near Lomax.	79.6	1952-68	1960	24	18.82	1,750 1,270	40		
2	South Loup River at Ravenna.	1,660 2 890	1941-58, 1968.	1947		12.6	(1)			
					24	10.32	17,100	³ 1.07		
3	South Loup River at St. Michael.	2,560 21,650	1943-68	1947	24	(¹) 11.00	(1) 27,500	³ 1. 2 5		

Not determined.
 Contributing area

FLOOD OF JULY 9 IN CANYON CREEK BASIN NEAR LOWMAN, IDAHO

By C. A. THOMAS

Intense flooding occurred in a few square miles of mountainous terrain along Canyon Creek near Lowman, Idaho (fig. 16), from precipitation associated with a thunderstorm of short duration which began about 6:30 p.m. on July 9. No precipitation data were available for the flood area, and only scattered light showers were recorded at nearby weather stations.

The resulting flood, although confined to a small area in a sparsely settled area, was outstanding owing to the unusually high unit discharges. An indirect measurement of the peak discharge in a small tributary to Canyon Creek determined that a discharge of 1,550 cfs was produced from a drainage area of only one-fourth of a square mile. The high flow lasted for about 1

² Included in amount for following day.
³ Of which 3.6 inches fell in 1 hour.

³ Ratio of peak discharge to 50-year flood.

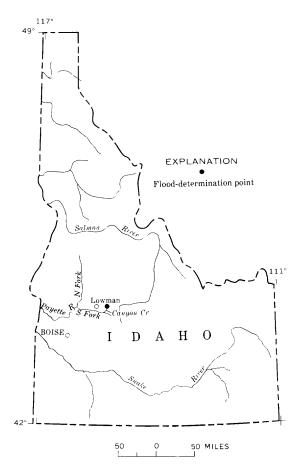


FIGURE 16.—Flood area; location of flood-determination point, flood of July 9 in Canyon Creek basin near Lowman, Idaho.

hour, and about 1 cfs was still flowing in the ephemeral channel on July 12, a notably long period for such a flow to continue.

The stream channel was severely eroded, and debris and gravel were deposited up to 6 feet deep on adjacent State Highway 21.

FLOODS OF JULY 16-20 IN NORTHEASTERN IOWA

By HARLAN H. SCHWOB

A severe storm occurred in the central part of northeastern Iowa on the night of July 16–17. The storm caused record floods

in the central part of the Wapsipinicon River basin and on tributaries of the Cedar River, in the Iowa River basin. Four lives were lost, three by drowning and one by electrocution. A flood-control dam on Virden Creek was overtopped and the outlet structure was washed out when the design flood was exceeded. Total damage was estimated at \$14.5 million.

Three areas had total rainfall of 14 or more inches during the storm. As shown in figure 17, these three areas were at Waverly, north of Waterloo, and north of Independence. The official amount of 12.40 inches in an observational day (July 17) at the Shell Rock rain gage was a record for northeastern Iowa. An unofficial 16.20 inches in Waverly, from a bucket survey, was the greatest 24-hour rainfall known in northeastern Iowa (U.S. Weather Bureau, 1968). Three recording rain gages were in the storm area although not in any of the specific areas of greatest total fall. Figure 18 is a graph of the accumulated rainfall at the gages for July 16 and 17. The graph shows that most of the rainfall was during a 12-hour nighttime period at the three widely separated gages.

Flood-peak data (table 14) were obtained at complete-record gaging stations, partial-record gaging stations, and at miscellaneous sites (fig. 17).

The Wapsipinicon River flood at the Independence gage (No. 5, table 14) was maximum for the period of record. The stage of 21.11 feet is the greatest known since at least 1901. Only about half of the drainage area of 1,048 square miles at the gage received 4 or more inches of rain. The tributaries of the Wapsipinicon River, principally the Little Wapsipinicon River and Crane Creek, were discharging extremely large amounts of water. Smaller, unmeasured tributaries were also contributing large amounts of water to the flooding river upstream from Independence.

The heavy rains near Independence produced record floods on Malone Creek at Independence (No. 6, table 14) and on Pine and Buffalo Creeks, 6 and 10 miles, respectively, east of Independence. These streams are tributary to the Wapsipinicon River downstream from the Independence gage. As the crests on these streams and on the Wapsipinicon River moved downstream, the peak discharge was attenuated by valley storage and the lack of sustaining flow from the lower tributaries. Near the mouth of Wapsipinicon River (No. 14, table 14) the peak discharge was that of a flood with a recurrence interval of only 2 years.

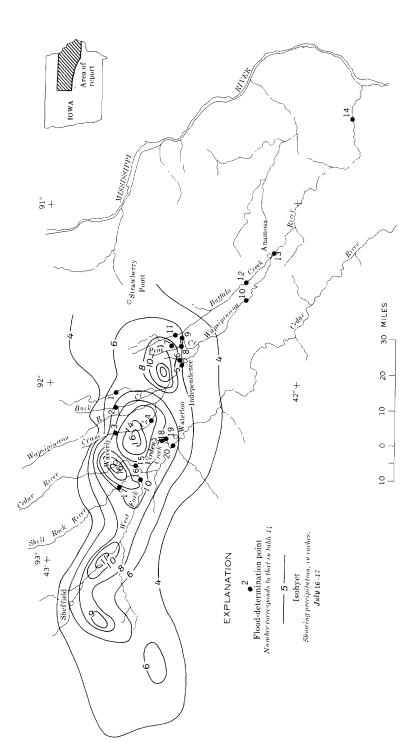


FIGURE 17.--Flood area; location of flood-determination points and isohyets for July 16-17, floods of July 16-20 in northeastern Iowa.

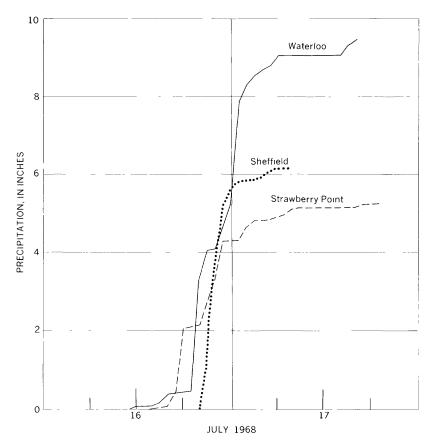


FIGURE 18.—Accumulated precipitation at three stations, floods of July 16-20 northeastern Iowa.

A relatively small part of the Cedar River basin was in the storm area. Consequently, floods on the major tributaries and the main stem were minor—generally less than a 10-year flood. However, smaller tributaries in the vicinity of Waterloo, notably Virden Creek, produced large and damaging floods. Two miscellaneous measurements (Nos. 18, 19, table 14) indicate the severity of the floods on this creek.

The Corps of Engineers determined the damages or costs caused by the flood in three cities. Floods on Malone Creek caused \$93,000 damage at Independence, and floods on Virden Creek caused \$950,000 damage at Waterloo. On Wapsipinicon River flood fighting at Anamosa reportedly cost \$20,000.

Table 14.—Flood stages and discharges, July 16-20 in northeastern Iowa

					Maxim	um floods		
			77 L				Disch	arge
No.	Stream and place of determination	Drainage area (sq mi)	Known b July 1 Period	968 Year	July - 1968	Gage height (feet)	Cfs	Recur- rence interva
								(years
		,	Wapsipinicon	River ba	sin			
1	Little Wapsipini- con River near Oran.	94.1	1966-68	1966	17	87.75 90.00	1,450 5,000	¹ 1.2
2	Buck Creek near Oran,	37.9	1966-68	1966	17	88.94 89.11	800 950	 <2
3	Crane Creek near Readlyn.	45.5			17	2 988.78	2,750	35
4	Crane Creek near Dunker- ton.	82.3		-	17	2 959.84	17,000	¹ 5.0
5	Wapsipinicon River at Independence.	1,048	1934-68	1947	18	18.74 21.11	21,500 26,800	- ī 1.0
6	Malone Creek at Independence.	10.0	1950	1950	17		4,910 10,700	>50
7	Pine Creek tributary near Winthrop.	.33	1953–68	1959	17	8.67 8.97	304 334	29
8	Pine Creek near Winthrop.	28.3	1950-68	1950	17	$21.70 \\ 22.98$	14,500 24.200	- 1 6.3
9	Pine Creek tributary at Winthrop.	.70	1953-68	1959	17	7.12 7.26	443 570	1 1.0
0	Wapsipinicon River at	1,263	1929, 1941–50	1947		² 835.24	22,500	
1	Central City. Buffalo Creek	68.2	1957-68	1960	19	² 829.53 18.59	23,000 5,350	20
	near Winthrop.	00.2	1301-00	1900	17	19.36	14,100	1 2.3
2	Buffalo Creek near Central City.	185	1964	1964	18	² 847.84 ² 848.58	3,600 4,000	3
.3	Wapsipinicon River near Anamosa.	1,575	1962, 63	1962	20	² 775.61 ² 777.90	15,400 19,000	8
4	Wapsipinicon River near DeWitt.	2,330	1935-68	1944	25	12.07 11.91	26,000 13,800	2
			Iowa Riv	er basin				
15	Cedar River at Janesville.	1,661	1905-06, 1915-21, 1923-27, 1929, 1933-42, 1945-68.	1961		16.33	37,000	-
16	West Fork Cedar	846	1945~68	1951	17	12.79	21,700	6
.0	River at Finchford.	540	1940~68	1991	19	17.28 14.51	31,900 10,600	3
7	Shell Rock River at Shell Rock.	1,746	1856, 1953–68.	1856		17.7	45,000	
8	Virden Creek near	8.64			17 17	10.68	4,040 6,640	<2 >50
9	Waterloo. Virden Creek tributary at Waterloo.	.58			17		811	>50
0	Waterloo. Cedar River at Waterloo.	5,146	1929, 1933,	1961		21.86	76,700	
			1941-68.		18	12.35	22,800	2

 $^{^1}$ Ratio of peak discharge to 50-year flood. 2 Elevation in feet above mean sea level (1929 adjustment).

FLOODS OF JULY 25 IN SANTA FE, NEW MEXICO

By E. L. HOGUE

On July 25 a flow of moist, unstable air from the Gulf of Mexico caused heavy precipitation, which was centered 2 miles northeast of downtown Santa Fe in the foothills of the Sangre de Christo Mountains. Intense precipitation was orographically influenced as is often the case with severe storms in the Santa Fe area. Most precipitation occurred between 1 p.m. and 5 p.m. The storm total of precipitation recorded by the U.S. Weather Bureau gage at Los Cerros Reservoir in the eastern part of Santa Fe was 3.51 inches; maximum observed precipitation (in a storage-type rain gage) was 4.0 inches; maximum recorded intensity was 2.08 inches per hour. Figure 19 shows the pattern and distribution of rainfall for the storm and the flood-determination points. The isohyets are based on bucket-survey readings obtained by employees of the U.S. Geological Survey, the U.S. Soil Conservation Service, and the office of the State Engineer of New Mexico.

The Santa Fe River flows westerly through the city and the channel is normally dry except during periods of snowmelt or storm runoff. Between Twomile Reservoir and downtown Santa Fe there are 6 square miles of contributing, unregulated drainage. Most of the flood flow came from Canada Ancha, a tributary that enters the Santa Fe River from the north, 1.4 miles downstream from Twomile Reservoir. During the storm period there was no significant outflow from Twomile Reservoir.

Most of the flood damage was confined to a narrow strip along the Santa Fe River between the mouth of Canada Ancha and the mouth of Arroyo Mascaras. Constriction of the channel by bridges and culverts augmented by a heavy accumulation of debris and vegetation in the channel forced flood water into adjacent streets along the right bank. Flooding was most severe in the business area immediately north of the Jefferson Street crossing. Flood water reached a depth of 4 feet in some buildings. Arroyo Mascaras also flooded Jefferson Street at the bridge crossing north of the business district, halting traffic for several hours.

A tabulation of peak discharges measured by indirect methods is shown in table 15. Flood frequency relationships have not been determined for this urbanized area. Previous floods occurred in 1872, 1904, 1914, 1921, 1929, and 1957. The flood of 1904, prior to the construction of Twomile Reservoir, destroyed three bridges in downtown Santa Fe.

A summary of flood damage compiled by the U.S. Army Corps of Engineers for the floods of July 25 is given below:

Type of damage	$Estimated\ damage$
Residential	\$ 35,000
Business	134,000
Utilities	12,000
Highways, streets, and roads	74,000
Flood control and drainage structures	127,000
Miscellaneous	15,000
Total	\$397,000

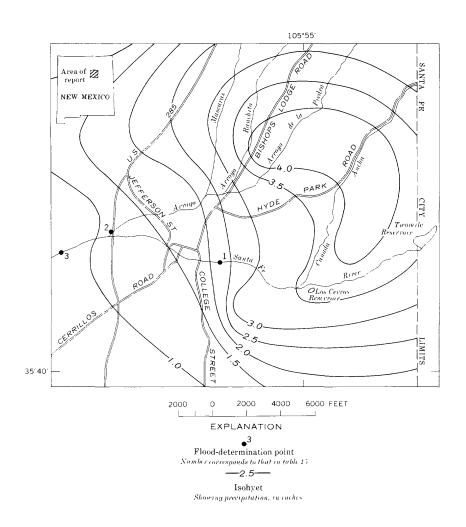


FIGURE 19.—Flood area; location of flood-determination points and isohyets for July 25 (from map furnished by the U.S. Army Corps of Engineers), floods of July 25 in Santa Fe, N. Mex.

			Maximum floods							
No.	Stream and place of determination	Drainage	Known before July 1968		July	Gage	Discharge			
		area (sq mi)	Period	Year	1968	height (feet)	Cfs			
1	Santa Fe River at Santa Fe, 1,500 ft upstream from College Street.	1 6.1	1952–57	1957	25		1,450 3,420			
2	Arroyo Mascaras at Santa Fe, 100 ft up- stream from mouth.	6.27	1957	1957	25		3,040 2,490			
3	Santa Fe River below Arroyo Mascaras at Santa Fe. 0.5 mi	¹ 13.9	-		25		6,110			

Table 15.—Flood stages and discharges, July 25 at Santa Fe, N. Mex.

downstream from Arroyo Mascaras.

FLOODS OF AUGUST 1-2 IN SOUTHEASTERN UTAH

By G. E. PYPER

Thunderstorms were frequent throughout Utah from July 21 to August 28, with most of the State receiving substantial amounts of precipitation. Fifteen precipitation stations in the State, two with records beginning in the 1890's, reported the wettest August of record. Many isolated floods occurred around the State but caused little damage.

Only the record-breaking rainfall and floods that occurred on August 1–2 in southeastern Utah are considered in this report. Hardest hit was the San Juan River basin. The area affected is shown in figure 20.

The severity of the rainfall and of the floods was unusual for Utah, and hence they were of considerable hydrologic interest. The floods were caused by an intense thunderstorm following 11 days of antecedent rainfall that left the upper soil zone near saturation; therefore, most of the cloudburst rainfall on August 1 became surface runoff.

The severe thunderstorm of August 1 centered on the mountains north of Bluff and Blanding and west of Monticello. The precipitation recorded at Bluff for August 1 was 0.40 inch for 24 hours, whereas the Monticello gage recorded 3.38 inches and the Blanding gage recorded 4.48 inches. New precipitation-intensity records at Blanding were recorded on August 1 for the following periods:

Length of period	Rainfall (inches)	Beginning of period
15 min	1.07	1:45 p.m.
30 min	1.83	9:42 a.m.
1 hr	2.05	9:42 a.m.
2 hrs	2.07	9:42 a.m.

¹ Contributing area.

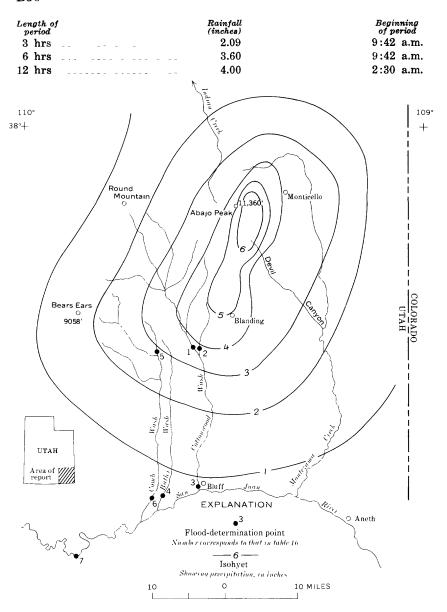


FIGURE 20.—Flood area; location of flood-determination points and isohyets for 24-hour precipitation, August 1-2, floods of August 1-2 in southeastern Utah.

A 24-hour catch of 4.59 inches at Blanding (recorded August 1-2), although not a record for the State, was more than 1.6 times as much as that of the U.S. Weather Bureau "100-year 24-

hour" storm at Blanding, and it was near the normal May-September (5-month) catch of 5 inches. The precipitation for a 24hour maximum May-October storm at Blanding for return intervals of 50 and 100 years is 2.5 and 2.8 inches, respectively (U.S. Weather Bureau, 1961).

The peak discharge of 42,100 cfs at the gaging station Cottonwood Wash at Bluff (No. 3, table 16), from a drainage area of 340 square miles, was more than 10 times the previous maximum in the 10 years of record. Twenty-one miles upstream, at Cottonwood Wash near Blanding (No. 1, table 16), the peak discharge of 20,500 cfs from 205 square miles was more than twice the previous maximum in the 10 years of record.

The recurrence interval at most stations exceeded 50 years. A summary of the flood stage and discharge for the points in figure 20 is shown in downstream order in table 16.

The area affected by flooding is very sparsely populated, and no lives were lost. Damage was limited to roads, bridges, and rangelands, except in Bluff where businesses, homes, and farmlands were damaged. Damage in Bluff was estimated by the U.S. Soil Conservation Service at \$33,700.

Although the precipitation was intense also in the area east of Monticello and Blanding (Montezuma Creek drainage), the flooding was not severe. The peak discharge in Montezuma Creek had a recurrence interval of less than 10 years.

Table 16.—Flood stages	and discharges,	August	1-2,	San	Juan	River	basin,
	south eastern	n Utah					

			Maximum floods							
		D !	TZ				Discharge			
No.	Stream and place of determination	Drainage area (sq mi)		Known before August 1968		Gage height		Recur-		
110.	or determination	(sq mi)	Period	Year	- 1968	(feet)	Cfs	rence interval (years)		
1	Cottonwood Wash near Blanding.	205	1959–68	1963	1	1 18.55 20.68	8,650 20,500	² 6.4		
2	Unnamed tributa- ry to Cotton- wood Wash near Blanding.	13.6	*		1		7,300	(3)		
3	Cottonwood Wash at Bluff.	340	1959-68	1963	i	4 15.24 18.38	4,060 42,100	² 11.8		
4	Butler Wash near Bluff.	54	1959-68	1963	1	16.10 14.87	1,000 820	(3)		
5	Comb Wash near Blanding.	10.3	1959-68	1964	ĩ	$12.80 \\ 14.48$	1,400 3,430	(3)		
6	Comb Wash near Bluff.	280	1959-68	1959	1	$\frac{3.32}{5.57}$	2,840 8,390	² 3.1		
7	San Juan River near Bluff.	23,000	1914-17, 1927-68.	1927	- •	32.0	70,000			
					2	14.06	17,500	2		

At site 300 ft upstream at different datum.
 Ratio of peak discharge to 50-year flood.
 Undefined by flood-frequency data but probably greater than 50-year flood.
 At site of old highway crossing at different datum.

FLOODS OF AUGUST 3 AND 4 IN GLOBE, ARIZONA

By S. C. Brown

On August 3 a storm cell centered over Globe, and 2.56 inches of rain fell at the U.S. Weather Bureau precipitation station in the downtown area (fig. 21) between 2:45 p.m. and 4:10 p.m. A resident about 0.6 mile northeast of the Weather Bureau gage reported 3.3 inches of rain in 45 minutes. Pinal Creek, choked with debris that had washed in from its tributary canyons, overflowed its banks and flooded a few stores and part of U.S. Highway 60 and 70 in downtown Globe; flooding in the northerly draining tributary canyons was severe.

A child was drowned in the rapidly rising water when he and his sister attempted to cross a street about a quarter of a mile below the confluence of Graveyard and Echo Canyons (No.

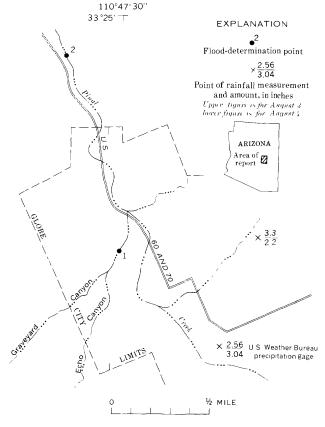


FIGURE 21.—Flood area; location of flood-determination points and rainfall-data sites, floods of August 3 and 4 in Globe, Ariz.

1, fig. 22); the peak discharge estimated from channel properties, was about 3,000 cfs from a drainage area of 1.2 square miles. A resident who had lived in Graveyard Canyon since 1933 described the flood as the greatest of any he had seen in the canyon. According to local reports, 16 automobiles were swept down Pinal Creek, bridge and culvert openings were choked with debris and some were overtopped, roofs were ripped from buildings by the high winds that preceded the rain, and electrical power was off for several hours on August 3 and 4. The Globe water system sustained about \$15,000 in damage, and a reported \$25,000 in damage occurred at a local automobile agency.

An indirect measurement of the peak discharge of Pinal Creek about half a mile north of the Globe city limits (No. 2, fig. 22) was calculated as 7,400 cfs from a drainage area of 34.4 square miles; the discharge has a recurrence interval of about 25 years. On July 29, 1954, a peak discharge of 8,130 cfs was measured at about the same site and on August 17, 1904, a peak discharge of 13,200 cfs was measured at a site nearby, where the drainage area was 33.4 square miles.

Another heavy rainstorm on August 4 began about 7:00 p.m. and 3.04 inches of rain was recorded at the U.S. Weather Bureau precipitation station. According to local observers, 2 inches of rain fell in the first hour. The rain gage 0.6 mile northeast of the Weather Bureau gage recorded 2.2 inches during the storm. Although Pinal Creek continued to flow on August 4, it did not overflow its banks.

FLOODS OF AUGUST 5-6 IN CEDAR RIVER BASIN, IOWA

By HARLAN H. SCHWOB

Heavy rainfall on the night of August 4–5 caused a flash flood in the Dry Run Creek basin (drainage area of 24.2 sq mi) in Cedar Falls, Iowa. The most rainfall reported was 7.44 inches at Parkersburg about 17 miles west of Cedar Falls. However, much heavier rainfall in a few hours was probable over the creek basin if unofficial estimates are considered. The severity of the resulting flood also tends to confirm this probability. Figure 22 shows the location of the creek, the gaging stations, and the rainfall amounts reported by the U.S. Weather Bureau.

Two indirect measurements of discharge were made in the Dry Run Creek basin. Both peak discharges were much greater than the estimate of the 50-year flood (table 17). Floods at regular gaging stations in the vicinity (Nos. 1, 4, 5, table 17) were comparatively small.

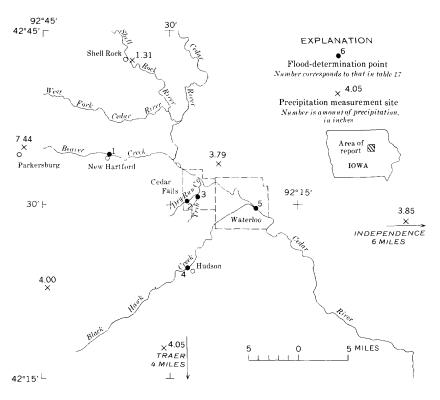


FIGURE 22.—Flood area; location of flood-determination points, floods of August 5-6 in Cedar River basin, Iowa.

Table 17.—Flood stages and discharges, August 5-6 in Cedar River basin, Iowa

			Maximum floods							
			771				Discharge			
	Channe and alone	Drainage	Known b August		August - 1968	Gage height	Cfs	Recur- rence		
No.	Stream and place of determination	area (sq mi)	Period	Year		(feet)		interval (years)		
1	Beaver Creek at New Hartford.	347	1946–68	1947	6	13.5 9.39	18,000 2,510	<2		
2	Dry Run Creek at Cedar Falls.	8.46			5	2 892.5	7,600	¹ 6.3		
3	Dry Run Creek tributary at Cedar Falls.	9.71			5	² 870.71	5,170	¹ 4.1		
4	Black Hawk Creek at Hudson.	303	1953-68	1960	6	16.93 15.26	9,000 3,470	-< 2		
5	Cedar River at Waterloo.	5,146	1929, 33, 1941-68.	1961		21.86	76,700			
	1, 4,01,001		1041-00.		5	9.64	13,800	<2		

¹ Ratio to 50-year flood.

² Elevation in feet above mean sea level (datum of 1929) at downstream side at bridge.

The flood elevations through Cedar Falls were determined after the flood peak. These data are shown in Table 18 and can be used to plot a flood profile along the reach of the creeks within the city.

One life was lost and considerable damage resulted from the floods. Several homes and small businesses were flooded, highways and streets were temporarily closed, and equipment from the city storage yard was washed into the creek. At one point on the tributary to Dry Run Creek a manhole on a sanitary sewer located under the creek bed was broken and raw sewage entered the creek until repairs were made.

Table 18.—August flood elevations, in feet above mean sea level, in Dry Run Creek basin, Cedar Falls, Iowa

		Eleva	tion
Mile	Place of determination	Downstream	Upstream
	Dry Run Creek		
0	Mouth of Dry Run Creek at Cedar River,		
	mile 194.9	- 852.4	
.05	C.R.I. & P. RR. bridge	- 852.8	853.0
.42	Waterloo Road bridge	- 859.8	860.3
.53	City storage yard	- 861.1	
.60	At end of 17th St.	862.8	
.72	Upstream of Rainbow Dr.		864.4
.83	Intersection 20th St. & Bluff St.	- 865.8	
1.15	Downstream of Seerley Blvd.	868.4	
1.19	Upstream of Seerley Blvd.		870.4
1.27	Main Street bridge	870.8	873.6
1.56	27th Street bridge (Highway 58)	₋ 876.5	877.9
2.40	At Panther Lane	889.7	
2.63	Highway 58 bridge (Hudson Rd.)	892.5	895.4
	Dry Run Creek tributary		
1.22	Tributary mouth	870.6	
1.40	Highway 218 bridge		872.9
1.86	Main Street bridge		
1.90	C.G.W. RR. bridge		881.4

FLOODS OF AUGUST 15 IN WESTERN NEBRASKA

By H. D. BRICE

As much as 6.40 inches of rain fell in the southeastern part of the Nebraska panhandle within a 12-hour period on August 15, according to U.S. Weather Bureau measurements (fig. 23). Even larger amounts—as much as 9 inches—were reported unofficially. Because the recurrence interval of a point rainfall of 4.5 inches in 12 hours in this part of Nebraska is 100 years (U.S. Weather Bureau, 1961), the recurrence interval of amounts as large as those recorded and reported would be much greater than 100 years.

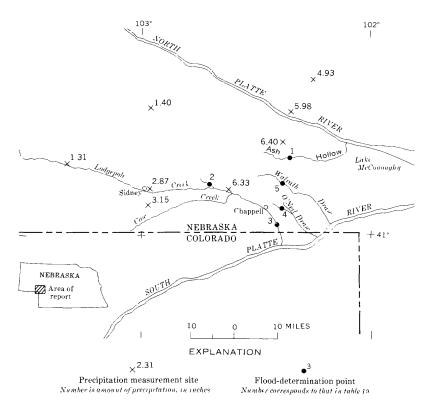


FIGURE 23.—Flood area; location of flood-determination points and rainfall data sites, floods of August 15 in western Nebraska.

Peak discharges were measured at five sites in the storm area (table 19). The peak on Lodgepole Creek at Ralton (No. 3, table 19) was about four times the previous maximum and had an estimated recurrence interval greater than 100 years. As flood-frequency relations have not been defined for streams in this area and no records of earlier peak discharges are available, recurrence intervals could not be estimated for the other four sites.

Reports of damages were most numerous from the vicinity of Chappell. U.S. Highway 30 near that town was under water and several sections of roadbed were eroded. South of Chappell, a gully 100 feet wide and 6 feet deep was incised across a 90-acre field. Also, many bridges and culverts in the area were washed out. Although a small dam west of Chappell gave way, four ranch dams north of town held and were credited with preventing serious flooding in the town. A few elderly area residents, whose memories spanned about 75 years, stated that they had

never experienced such a severe storm nor seen such great flood damage before.

No.	Stream and place of determination	Drainage area (sq mi)	Maximum floods						
			Known before August 1968			_	Discharge		
					August	Gage height		Recur-	
			Period	Year	1300	(feet)	Cfs	interval (years)	
1	Ash Hollow near Oshkosh,	48.4			15		3,440		
2	Tributary to Lodgepole Creek near Lodgepole.	15.8			15	-	820	-	
3	Lodgepole Creek at Ralton.	3,307	1931, 1951–68	1951	15	$5.70 \\ 6.49$	1,150 4,560	100	
4	Tributary to O'Neil Draw near Chappell.	2.07			15		302		
5	Walrath Draw near Chappell.	69.5			15		2,200		

FLOODS OF AUGUST 28-29 IN THE TEXAS HIGH PLAINS

By L. G. STEARNS

Heavy rains of up to 8 inches fell on Prairie Dog Town Fork Red River basin (fig. 24) between Canyon and Estelline during the afternoon and night of August 28. On August 28, as a result of these heavy rains, a \$2 million train wreck 15 miles northwest

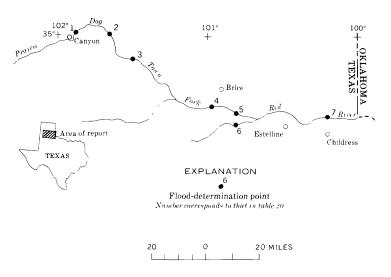


FIGURE 24.—Flood area; location of flood-determination points, floods of August 28-29 in the Texas High Plains.

of Childress claimed one life. The derailment occurred at a point where the railroad crosses a small draw.

The U.S. Weather Bureau reported 7.87 inches of rain at Canyon, and all highways through Canyon were closed for a time during and immediately after the storm. A peak stage of 20.54 feet (determined from drift marks) occurred at the discontinued stream-gaging station Prairie Dog Town Fork Red River near Canyon (No. 1, table 20). Above Palo Duro State Park (No. 2, table 20) survey levels to drift at the slope-area site show that the crest of the flood was about 1.5 feet lower than the flood of June 1960.

The maximum discharges at gaging stations 2 and 3 in table 20 were the most outstanding observed in the basin during this period of flooding.

Table 20.—Flood stages and discharges, August 28-29 in the Red River basin, Texas High Plains

No.	Stream and place of determination	Drainage area (sq mi)	Maximum floods					
			Known before August 1968				Discharge	
					August		-	Recur-
			Period	Year	- 1968	height (feet)	Cfs	rence interval (years)
1	Prairie Dog Town Fork Red River near Canyon.	3,369 1 711	1905-68	1951	28	20.31 20.54	15,200 15,500	14
2	Prairie Dog Town Fork Red River above Palo Duro State Park near Canyon.	1 743	1951-68	1960	28		52,700 48,000	2 1.6
3	Prairie Dog Town Fork Red River near Wayside.	4,211 1 930	. w = _		28	13.0	58,000	² 1.7
4	Prairie Dog Town Fork Red River near Brice.	6,082 1,581	1906-68 1958-68	1957 1960	28	$^{19.2}_{12.20}_{12.6}$	(3) 49,000 54,000	14
5	Prairie Dog Town Fork Red River near Lakeview.	6,792 1 2,023	1963-68	1965	29	10.50 9.10	37,900 51,000	(3)
6	Little Red River near Turkey.	139			2 8	13.48	3,570	3
7	Prairie Dog Town Fork Red River near Childress.	7,725 1 2,956	1899-1968	1957	29	16.9 11.15	(³) 38,500	(3)

¹ Contributing area.

FLOODS OF AUGUST 28-31 IN THE VICINITY OF **JACKSONVILLE, FLORIDA**

By JAMES W. RABON

A low-pressure disturbance which formed over central Florida on August 27 drifted aimlessly for several days before moving northward into Georgia at the end of the month. Resulting rains were especially heavy in Duval and Clay Counties (fig. 25), where

² Ratio of peak discharge to 50-year flood.

Not determined.

considerable flooding occurred on small streams having drainage areas of less than 200 square miles.

The U.S. Weather Bureau at Imeson Airport, Jacksonville (fig. 25), recorded 15.17 inches of rainfall during the 4-day storm period of August 28–31, 7.82 inches of which fell on the 28th, 2.73 inches on the 29th, 3.88 inches on the 30th, and 0.74 inches on the 31st. The recurrence interval of the 4-day storm rainfall at this point is about 100 years. Major damage resulted in the upper Cedar Creek basin where many houses and commercial establishments were flooded or were surrounded by flood waters, and 11 bridges were inundated to depths from a few inches to 4.5 feet (U.S. Army Corps of Engineers, 1969).

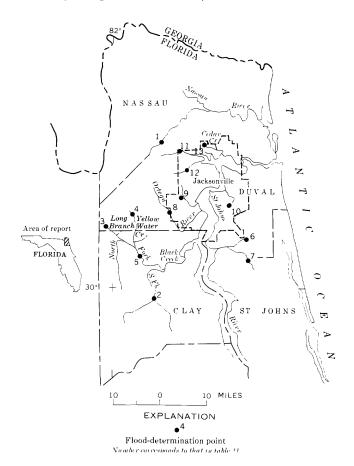


FIGURE 25.—Flood area; location of flood-determination points, floods of August 28-31 in the vicinity of Jacksonville, Fla.

Long-term flood record on streams in the Jacksonville urban area and Duval County are scarce. (See table 21.) The U.S. Geological Survey obtained highwater marks after two notable floods in May 1964 and in September 1964 (Hurricane Dora) and installed some crest-stage gages. The U.S. Army Corps of Engineers made additional field investigations, searched newspaper records, and interviewed local residents to supplement the data. Flood records for two long-term stations on the North Fork (No. 5, table 21) and South Fork (No. 2, table 21) of Black Creek, including data for the historic floods of 1919 and 1944, were used as references in determining relative frequencies for the August

Table 21.—Flood stages and discharges, August 28-31 in vicinity of Jacksonville, Fla.

	Stream and place of determination	Drainage area (sq mi)	Flood peaks					
No.							Discharge	
					-August			Recur- rence
			Period	Year	1968	sea level)	Cfs	interval (years)
	Coastal ba	sins between	St. Marys R	iver and	St. Joh	ns River.		
1	Thomas Creek near	29.8	1950	1950		23.3	(¹)	
	Crawford.		1965-68	1966	31	18.97 19.49	$1,610 \\ 1.920$	(¹)
		St.	Johns River					
2	South Fork Black	134	1940-68	1944		36.15	13,900	
2	Creek near Penney	104	134000	² 1964		26.30	3,090	
	Farms.			3 1964	31	29.86 30.44	6,440 7,080	6
3	Long Branch at Maxville.	2.98	1965-66	1965	30	86.93 87.90	294 380	22 22
4	Yellow Water Creek near Maxville.	25.7	1958–68	² 1964 ³ 1964	29	59.81 58.97 59.27	3,220 1,890 2,520	24
5	North Fork Black Creek near Middle- burg.	174	1919 1932–68	1919 2 1964 3 1964		25.9 24.53 23.54 23.94	15,000 12,000 11,200 11,800	14
6	Big Davis Creek at Bayard.	13.6	1964-68	3 1964	29	10.05 10.47	780 1,170	11
7	Durbin Creek near Durbin.	36.7	1961-68	² 1964 ³ 1964	30	6.78 10.10 10.07	1,350 4,140 4,050	50
8	Ortega River near Jacksonville.	27.8	1964-68	² 1964	30	38.28 39.32	1,670 2,950	35
9	Wills Branch near Marietta.	7.04	1964-66	³ 1964	30	$10.38 \\ 10.55$	791 820	23
10	Pottsburg Creek near South Jacksonville.	9.89	1964-66	³ 1964	$\bar{\tilde{30}}$	$\frac{10.09}{8.95}$	$1,740 \\ 1,250$	42
11	Trout River at Dins- more.	19.9	1961-68	² 1964 ³ 1964	29	7.13 7.54 9.62	564 646 1,140	4
12	Sixmile Creek near Marietta.	19.0	1964-66	3 1964	3 <u>0</u>	8.63 11.60	1,090 2,750	4 1.2
13	Cedar Creek near Panama Park.	12.0	1964-66	² 1964 ³ 1964	 30	5.67 5.91 8.39	239 282 1,250	20

¹ Unknown.

May; a non-tropical storm.
 September; Hurricane Dora

⁴ Ratio of peak discharge to 50-year flood.

28–31 floods. A comparison of basin characteristics for streams in the Jacksonville area with those of North and South Forks of Black Creek indicated that runoff characteristics were probably similar, particularly with respect to basin and channel slopes. one possible exception is Thomas Creek (No. 1, table 21), head of Nassau River basin, whose characteristics may be more comparable to those of St. Marys River basin, particularly with respect to basin storage.

Recurrence intervals for the August 28–31 flood peaks ranged from about 4 years to more than 50 years. Unit peak discharges ranged from 53 cfs per sq mi to 145 cfs per sq mi (No. 12, table 21). In the southern part of Duval County peak discharges at four stations, 7, 8, 10, and 12, averaged 122 cfs per sq mi; that at Sixmile Creek (No. 12, table 21) was 145 cfs per sq mi and 1.2 times the 50-year discharge. Table 21 and figure 26 may be used to compare the August 28–31 floods with the two outstanding floods of 1964. Storm rainfall associated with each of the 1964 floods was considerably less than that for the August 28–31 storm. However, soil moisture and general streamflow conditions preceding the 1964 storms were much more conducive to producing floods than antecedent conditions associated with the August floods. For example, cumulative runoff preceding the 1964 floods at long-term station 5 was

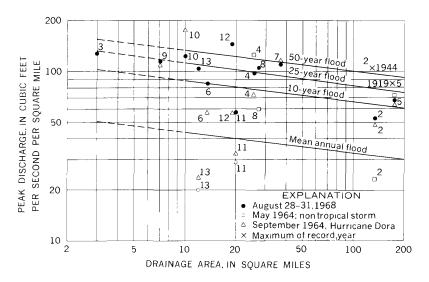


FIGURE 26.—Relation of August 28-31 flood peaks to major flood peaks of record in the vicinity of Jacksonville, Fla. Numbers correspond to flood determination points in table 21.

about four times normal for that time of year, while cumulative runoff preceding the August 1968 floods was only about $1\,1/2$ times normal. In fact, the northern and northwestern part of the State was experiencing a moderate drought until the rains in late spring and summer.

Peak stages at four stations, Nos. 2, 7, 8, and 11, ranged from about 1 to 4 feet higher than those of May 1964. Peak stages of August 1968 at six stations, Nos. 2, 4–7, and 9, ranged from about the same to about half a foot higher than those resulting from Hurricane Dora (September 1964); peak stages at three stations, Nos. 11–13, were about 2 to 3 feet higher. At one station, No. 5, the August peak stage was 2 feet lower than the maximum known stage which occurred in 1919 and the peak discharge was only 20 percent less than that of 1919.

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